#### **Civil Certification Pty Ltd**

Accredited Certifiers Civil Engineering

53 Werona Avenue Gordon, NSW, 2072 0412 264 237



ABN 56 607 721 595

Ir375mjs-20-9-21-Brunslea Park FH-da smp - (v2 final).doc

Wakefield Ashurst Developments Pty Limited c/- Yarraman Developments Pty Limited PO Box 3748 Marsfield NSW 2122

20 September 2021

#### Attention: Mr Brent Annis Brown

Dear Brent,

#### BRUNSLEA PARK, FOREST HILL APPROX. 500 LOT RESIDENTIAL SUBDIVISION CONCEPT STORMWATER MANAGEMENT PLAN (*SMP*)

#### **1. INTRODUCTION**

Civil Certification Pty Ltd has been engaged by Yarraman Developments Pty Ltd to prepare a concept Stormwater Management Plan (*SMP*) for the proposed 500 lot residential subdivision at the above site.

The aim of this report is to demonstrate how a stormwater treatment & detention strategy that meets the requirements of the City of Wagga Wagga Council and aligns with best practice WSUD can be achieved.

This report is in support of the "*Planning Proposal*" (*PP*) for the proposed subdivision. It should be read in conjunction with all other PP documentation.

#### 2. SITE

The site is illustrated in **Diagram 1** and is located on the west side of Wagga Wagga Airport. It has an area of approximately **156ha** and is broadly split into two catchments (*i.e. the Northern and Southern Catchments*). Both catchments drain to the west and eventually join the Murrumbidgee River.

The site currently contains cleared rural land with some limited patches of large eucalypt trees. An existing residential subdivision is located to the north and north east of the site

The site is proposed to be developed as part of future DAs to incorporate approximately 500 lots, new local streets, parks and all associated servicing / stormwater drainage infrastructure (*refer to Appendix A* for proposed development layout & land use).





**Diagram 1 – Site Locality** 

## 3. CATCHMENTS

The site is located downstream of a number of external catchments. To the west is an 50ha catchment containing parts of the airport. To the south is a 2,500ha catchment serving Gregadoo Creek. An illustration of the catchments in the vicinity of the site is contained in **Figure 2**.

The site itself is divided by a ridge running east west along the "*Active Travel Link*". Areas to the north of the ridge (*i.e. the Northern Catchment*) drain north and join Marshalls Creek which runs to the west. Areas to the south of the ridge (*i.e. the Southern Catchment*) drain south and join Gregadoo Creek which also runs to the west.

For modelling purposes the site catchment has be broken down into a number of sub-catchments (*i.e. A to H*). An illustration of these sub-catchments is contained in **Figure 1**.

## 4. STORMWATER/WSUD TREATMENT STRATEGY

A stormwater treatment strategy has been formulated for the proposed future development as illustrated in the **Figures** section at the rear of this report and as summarised below:

- Rainwater tanks on all future lots to capture roof runoff and reuse for toilet flushing, cold water laundry and garden irrigation;
- A rehabilitated riparian corridor running through the southern part of the site;
- Gross Pollutant Traps (GPTs) to treat both lot and road runoff (eight in total); and
- Community based bio-retention/detention basins (seven in total).



These measures will control urban generated pollutants at the source and minimise the export of suspended solids, nutrients and litter from the site.

The software package developed by the CRC for Catchment Hydrology termed "MUSIC" (*Model for Urban Stormwater Improvement Conceptualisation*) was used to assess the effectiveness of the proposed site based component of the "*treatment train*".

Details of the MUSIC modelling exercise (*including results*) are included at **Appendix B** and summarised in the following sections.

#### **4.1 TREATMENT TARGETS**

The City of Wagga Wagga Council stipulates the following treatment targets for this locality (*i.e., post development average annual load reductions from the CSIRO 1999 BPEMG*):

- Gross Pollutants
- Total Suspended Solids
- Total Phosphorous
- Total Nitrogen

70% reduction;80% reduction;45% reduction; and45% reduction.

#### 4.2 MUSIC

MUSIC is a continual-run conceptual water quality assessment model originally developed by the Cooperative Research Centre for Catchment Hydrology (*CRCCH*). MUSIC can be used to estimate the long-term annual average stormwater volume generated by a catchment as well as the expected pollutant loads. It is able to conceptually simulate the performance of a group of stormwater treatment measures (*treatment train*) to assess whether a proposed strategy is able to meet specified water quality targets.

MUSIC was chosen for this investigation because it has the following attributes:

- It can account for the temporal variation in storm rainfall throughout the year;
- Modelling steps down to 6 minutes to allow accurate modelling of treatment devices;
- It can model a range of treatment devices;
- It can be used to estimate pollutant loads at any location within the catchment; and
- It is based on logical and accepted algorithms.

#### **4.3 MUSIC INPUTS**

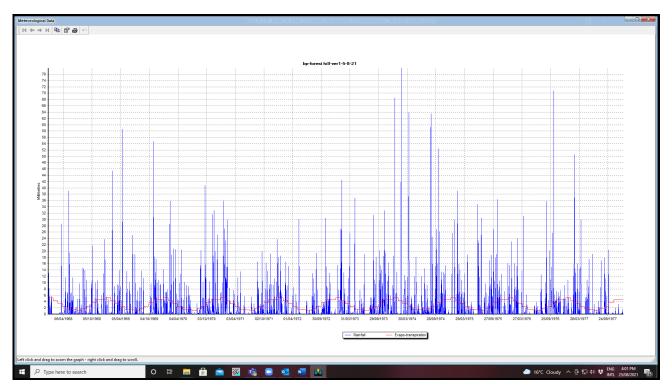
All MUSIC inputs were selected based on the site's locality, physical characteristics and BPEMG guidelines.

#### Rainfall and Evaporation (Climate Template)

Rainfall and evaporation data adopted in the MUSIC modelling exercise was sourced for this region and contains 6 minute rainfall data over a period of 10 years (*1968 to 1977 inclusive*) resulting in a mean annual rainfall value of 655mm and mean annual evapotranspiration value of 1,116mm. The period contains a mix of dry, average and wet years.



The long term annual average rainfall for the region is approximately 571mm (*i.e., sourced from BOM Station 07215 – Wagga Wagga AMO*).



Refer to **Diagram 2** for an illustration of the adopted Climate Template data.

#### **Diagram 2 – Climate Template**

#### **Sub Catchment Areas**

The site was assessed under proposed conditions and divided into 2 broad catchments (*i.e., north and south draining*). Each catchment was then further subdivided to represent the proposed land uses.

The proposed impervious fraction adopted for each sub-catchment was based on land-use and open space. General Residential land-use was assumed to be 65% impervious, Rural Residential  $(2,000m^2)$  land-use was assumed to be 50% and Rural Residential  $(4,000m^2)$  land-use was assumed to be 40%.

The existing catchments were assumed to be 2% impervious. The new rehabilitated riparian corridor was also assumed to be 2% impervious.

All developed sub-catchment areas are proposed to be served by both GPTs and bioretention/detention basins, except catchment G which is served by a GPT only.

Details of the sub catchment area characteristics are provided in **Table 1**.



Sub Catchment	Area (ha)	Land-use	% Impervious		
Proposed Conditions					
A	36.6	Rural Res. (2,000m <sup>2</sup> lots) & Light Industrial	50%		
B1	8.0	Rural Res. ( <i>4,000m<sup>2</sup> lots</i> )	40%		
B2	8.5	Rural Res. ( <i>4,000m<sup>2</sup> lots</i> )	40%		
С	12.8	Rural Res. ( <i>4,000m<sup>2</sup> lots</i> )	40%		
D	8.8	Rural Res. (2,000m <sup>2</sup> lots)	50%		
RIP	15.7	Rehabilitated Riparian Corridor	2%		
South Total	90.4ha	-	38.4%		
E	14.5	General Res. & Open Space	65%		
F	30.2	General Res. & Open Space	65%		
G	2.0	General Res.	65%		
H (Stage 1)	18.6	General Res.	65%		
North Total	65.3	-	65%		
Total Proposed Site	155.7ha	- 49.0			

#### Table 1 – Sub catchment Characteristics

#### Soil Data / Characteristics

A summary of the adopted soil parameters is contained in **Table 2**. These values have been derived based on site conditions, the *CSIRO 1999 BPEMG* and the NSW MUSIC Modelling Guidelines, August 2015.

#### Table 2 – Adopted Soil Data

	Units	Value
Impervious Area Parameters		
Rainfall threshold	mm/day	0
Pervious Area Parameters		
Soil storage capacity	mm	40
Initial storage	% of capacity	20
Field capacity	mm	25
Infiltration capacity coefficient – a		200
Infiltration capacity coefficient – b		1
Groundwater Properties		
Initial depth	mm	1
Daily recharge rate	%	25
Daily base flow rate	%	1
Daily deep seepage rate	%	15

#### **EMC Values**

A summary of the adopted EMC values is contained in **Table 3**. These values have been derived based on the site conditions, proposed development, *CSIRO 1999 BPEMG* and the NSW MUSIC Modelling Guidelines August 2015.



#### Table 3 – EMC Values

	Base Flow						Storm Flow					
	Т	SS	Т	Ρ	Т	N	TS	SS	Т	P	Т	N
	Mn	SD	Mn	SD	Mn	SD	Mn	SD	Mn	SD	Mn	SD
Land use		(all values expressed as log <sub>10</sub> mg/l)										
Urban Gen&RR	1.40	0.17	-0.82	0.19	0.32	0.12	2.19	0.32	-0.65	0.25	0.427	0.19
Riparian Corridor	0.90	0.13	-1.50	0.13	-0.14	0.13	1.90	0.20	-1.10	0.22	-0.08	0.24

# 4.4 PROPOSED TREATMENT TRAIN

An illustration of the MUSIC network constructed to represent the site under developed conditions is contained at **Diagram 3**. All new developed sub-catchments are treated by a GPT in series with large offline bio-retention/detention basins, apart from catchment G which is served by a GPT only.

The GPT provides primary screening to remove litter and coarse sediment, whilst the Bio-Retention basins provide tertiary treatment to remove fine sediments and nutrients.

Lot based measures (*i.e., rainwater tanks*) and the additional treatment provided in the proposed rehabilitated riparian model were conservatively omitted from the model.

Further details of the individual components of the proposed treatment train are provided below and in **Appendix B**.

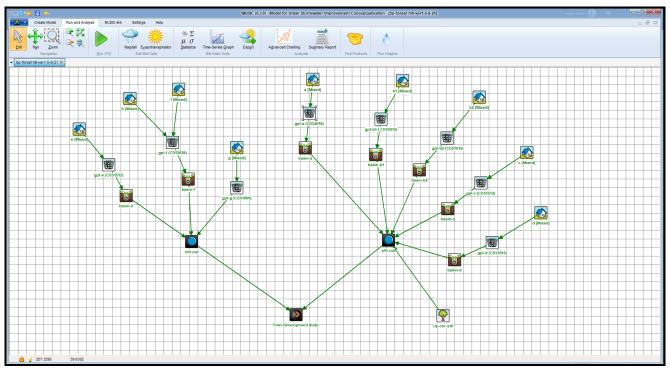


Diagram 3 - Proposed MUSIC Network Diagram



#### **Rainwater Tanks**

A minimum rainwater tank volume of 3KL/ future lot in the proposed future "*General Residential*" land-use areas and 10KL for the proposed future "*Rural Residential*" land-use areas will be adopted for subdivision. These tanks will capture runoff from all roof areas and reuse for garden *irrigation, cold water laundry and toilet flushing*. For the proposed future "*General Residential*" land-use areas, the anticipated reuse rate will be approximately 200-250 L/lot/day.

Rainwater tanks provide an indirect treatment mechanism by reducing the volume of water that is needed to be treated further downstream.

Flows in excess of the 1yr ARI are likely to bypass the rainwater tanks.

#### GPTs

Gross Pollutant Traps (*GPTs*) are proposed at the inlets of all proposed community bioretention/detention basins. These GPTs will treat all lot and road based runoff, primarily targeting coarse sediment and litter.

For this concept report, CDS GPT's have been modelled for each sub-catchment and sized to treat the 3-6month ARI storm. Equivalent proprietary products are available.

A summary of the GPT sizes adopted in our MUSIC model are included in Table 4.

Sub Catchment / GPT Name	Area (ha)	CDS Model
A	36.6	CDS2018
B1	8.0	CDS1015
B2	8.5	CDS1015
С	12.8	CDS1512
D	8.8	CDS1015
E	14.5	CDS1512
F&H	48.8	CDS2028
G	2.0	CDS1009
Total Dev A	140ha	-

#### Table 4 – GPT Sizing

#### **BIO-RETENTION BASINS**

A total of seven large community or regional style basins are proposed as part of the future development. These basins will act as both water quality treatment and stormwater detention facilities. All basins will be located offline and contain a bio-retention component.

The bio-retention system will consist of an above ground depression containing landscaping of native grasses & shrubs underlain by an infiltration zone and associated under drain. The infiltration zone would consist of a sandy loam topsoil layer, a filter media layer, a gravel transition layer and a subsoil drainage system at the base to collect filtered water. The primary treatment mechanisms for the system are detention/settling at the surface, take up of nutrients by plants, filtering treatment through the media and biological treatment from algal growth on the filter media.



A summary of the proposed bio-retention basin properties is included in **Table 5**.

Basin Name	Surface Area (m <sup>2</sup> )	Filter Area (m <sup>2</sup> )	Filter depth (mm)	Extended Detention Depth (mm)
А	9,100	4,550	1,000	200
B1	2,000	1,000	1,000	200
B2	2,500	1,250	1,000	200
С	3,200	1,600	1,000	200
D	2,200	1,100	1,000	200
E	3,600	1,800	1,000	200
F (H)	12,000	6,000	1,000	200
Total	34,600	17,300	-	-
%Dev A	2.5%			

# Table 5– Bio-Retention Basins

#### **4.5 MUSIC MODELLING RESULTS**

The MUSIC modelling results are detailed in Appendix B and summarised in Table 6.

The results illustrate that implementation of the proposed treatment strategy achieves in excess of the *CSIRO 1999 BPEMG* and best practice treatment targets.

#### Table 6 – MUSIC Modelling Results

		Annual Flow and Pollutant Load Results						
MUSIC model	Location	Flow	TSS	TP	TN	GP		
		(ML/yr)	(kg/yr)	(kg/yr)	(kg/yr)	(kg/yr)		
Proposed (Combine	d Receiving Node)							
	All Source Nodes	564	112,000	150	1,620	21,500		
	Residual Load	495	2,110	59	347	20.5		
% Effectiveness		12.2	98.1	60.6	78.6	99.9		
Achieve Target			80	45	45	70		
			YES	YES	YES	YES		
Proposed (South Dr	aining Catchment)	-	-			-		
	All Source Nodes	267	52,300	68.8	766	10,300		
	Residual Load	230	883	26.8	153	13.6		
% Effectiveness		13.9	98.3	61	80	99.9		
Achieve Target			80	45	45	70		
			YES	YES	YES	YES		
Proposed (North Dr	aining Catchment)							
	All Source Nodes	297	59,400	81.2	858	11,200		
	Residual Load	266	1,230	32.2	194	6.88		
% Effectiveness		10.7	97.9	60.3	77.4	99.9		
Achieve Target			80	45	45	70		
			YES	YES	YES	YES		



# 4.6 CONSTRUCTION STAGE WATER QUALITY

As part of the future development application for the proposed development, a detailed erosion and sediment control plan would be developed for the site in accordance with Council's guidelines and the NSW Blue Book (*NSW DECC publication titled "Managing Urban Stormwater – Soils and Construction" January 2008*).

The erosion and sediment control plan will outline the strategies proposed to prevent excessive pollutant loads being exported from the site in runoff during and immediately following construction (*i.e., primarily as a result of erosion caused by construction activity*).

A summary of the principal elements of a preferred erosion and sediment control plan for the site is summarised below:

- Minimising the extent of disturbed surfaces at any one time (*i.e., staging of earthworks etc*);
- Stabilising disturbed surfaces immediately upon completion of works (*i.e., hydromulch or vegetation*);
- Diverting clean runoff around disturbed work areas (*i.e., using earth bunds/diversion mounds/channels*);
- Protecting stockpiles (*i.e., using silt fence, diversion bunds, temporary vegetative cover etc*);
- Implementation of dust control/suppression measures during works(*i.e., perimeter fencing, wind velocity monitoring, cessation of earthworks activities during high wind conditions, watering down disturbed areas, setup of recycled water irrigation sprays etc)*;
- Use of sediment basins;
- Use of silt fencing downslope of disturbed surfaces;
- Use of silt socks or equivalent around existing drainage structures;
- Use of rock /haybale/mulch check dams along designated overland flow paths;
- Protection of exposed slopes;
- Restriction of vehicle entry/exit points to construction zones;
- Setup of stabilised site access points; and
- Setup of vehicle washdown/wheel wash baths at exit points of disturbed areas.

## **5. STORMWATER DETENTION STRATEGY**

A detention strategy has been formulated for the proposed future subdivision to mitigate the increase in flows generated by the change in land-use.

The strategy is illustrated in the **Figures** section at the rear of this report and consists of a number of large community based detention basins located near the outlet of each sub-catchment.

The proposed detention system has been designed to cater for all sub-catchments. Whilst subcatchment G does not drain directly to a basin the nearby Basin F has been oversized to ensure the net impact is zero.



# 5.1 RAFTS

RAFTS has been used to determine the quantity of detention required for the proposed development. Both the pre and post development site conditions were modelled for the 5yr ARI, 20yr ARI and 100yr ARI design storm events.

All sub-catchments were included in the model, including the Stage 1 sub-catchment H as this will ultimately replace be served by Basin F.

#### **5.2 RAFTS PARAMETERS**

#### IFD

The adopted IFD coefficients for the site are included in **Table 7**.

#### Table 7 – IFD Data

Duration	2 year ARI	50 year ARI
1hr	21.63	43.50
12hr	4.00	7.01
72hr	1.03	1.73

#### **Rainfall Losses**

The adopted rainfall losses for the site are included in **Table 8**.

#### Table 8 – Rainfall Losses

Surface	Initial Loss	Continuing Loss
Pervious	21.3mm	2.5mm/h
Impervious	1.0mm	0mm/h

#### **Other Assumptions**

A split sub catchment approach was adopted (*i.e., separate routing of pervious and impervious surfaces of the same sub catchment*). For conservatism zero lags were adopted in all links.

#### **5.3 RAFTS MODEL SCENARIOS**

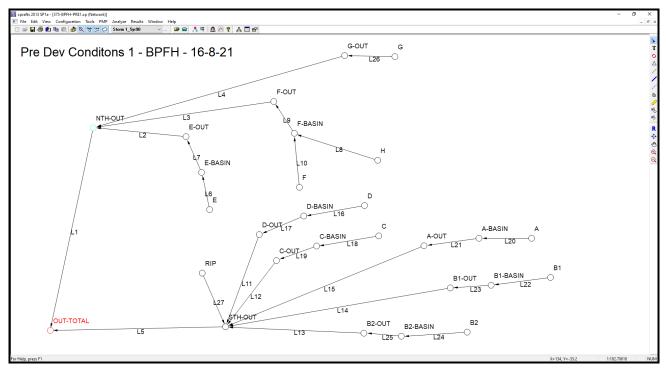
#### **Pre Development Conditions**

The adopted pre development condition catchment characteristics for the site are included in **Table 9**.

#### Table 9 – Pre development Catchment Characteristics

Parameters	North Catchments (E,F,G,H)	South Catchments (A,B1,B2,C,D)
Area (ha)	65.3	90.4
Vectored Average Slope (%)	2	2
Impervious Fraction (%)	2	2
Pervious Mannings "n"	0.045	0.045
Impervious Mannings "n"	0.025	0.025





A RAFTS network was constructed to represent the site under pre development conditions as illustrated in **Diagram 4**.

## **Diagram 4 – Pre Development Condition RAFTS Network**

The results of the pre development conditions RAFTS model are summarised in **Table 10**. Details of the model are included at **Appendix C**.

The RAFTS results show that under pre development conditions the critical storm duration is generally 120 minutes. The total peak flow discharged from the site under a 100 year ARI storm event is **9.20 m<sup>3</sup>/s**. An illustration of the critical 100yr ARI pre development condition site hydrograph is included at **Diagram 5**.



Storm Frequency	Storm Duration (min)	G- OUT	F- OUT	E- OUT	NTH- OUT	D- OUT	C- OUT	A- OUT	B1- OUT	B2- OUT	RIP	STH- OUT	OUT- TOTAL
100yr ARI	45	0.17	2.07	0.74	2.95	0.53	0.68	1.40	0.49	0.52	0.49	4.09	7.04
100yr ARI	60	0.19	2.60	0.91	3.67	0.62	0.82	1.78	0.57	0.60	0.62	4.98	8.65
100yr ARI	90	0.18	2.81	0.92	3.85	0.61	0.83	1.98	0.57	0.60	0.71	5.17	9.02
100yr ARI	120	0.17	2.88	0.94	3.94	0.61	0.85	2.03	0.56	0.59	0.74	5.28	9.20
100yr ARI	180	0.15	2.53	0.81	3.42	0.53	0.74	1.84	0.49	0.52	0.72	4.53	7.94
20yr ARI	90	0.09	1.34	0.45	1.84	0.30	0.40	0.92	0.28	0.29	0.31	2.45	4.28
20yr ARI	120	0.09	1.45	0.49	2.98	0.32	0.43	1.02	0.29	0.31	0.35	2.63	4.60
5yr ARI	90	0.03	0.45	0.16	0.64	0.12	0.15	0.30	0.11	0.11	0.11	0.89	1.53
5yr ARI	120	0.04	0.57	0.20	0.80	0.14	0.18	0.40	0.13	0.13	0.15	1.08	1.88

Note 1. Values shown in bold are critical.

2. Model 375BPFH-Pre1.xp



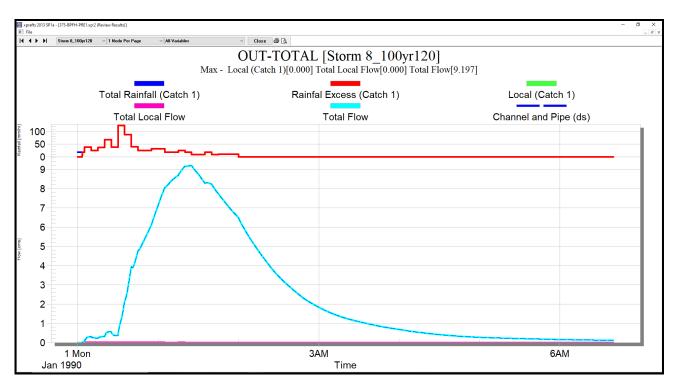


Diagram 5 – Pre Development 100yr ARI Hydrograph

## **Post Development Conditions**

To assess the impact of modifying the pre development land-use of the site (*i.e., increasing the impervious areas*), a proposed condition RAFTS model was established.

The adopted impervious fraction for the developed site catchments averaged 50% impervious (*varies from 40% to 65%*). The vectored average slope was maintained as per pre developed conditions.

A summary of the adopted post development condition catchment characteristics is included in **Table 11**.

Parameters	North Catchments (E,F,G,H)	South Catchments (A,B1,B2,C,D)
Area (ha)	65.3	90.4
Vectored Average Slope (%)	2	2
Impervious Fraction (%)	65%	40-50%
Pervious Mannings "n"	0.02	0.02
Impervious Mannings "n"	0.04	0.045

Table 11 – Post Development	Catchment Characteristics
-----------------------------	---------------------------

A RAFTS network was constructed to represent the site under post development conditions as illustrated in **Diagram 6.** 



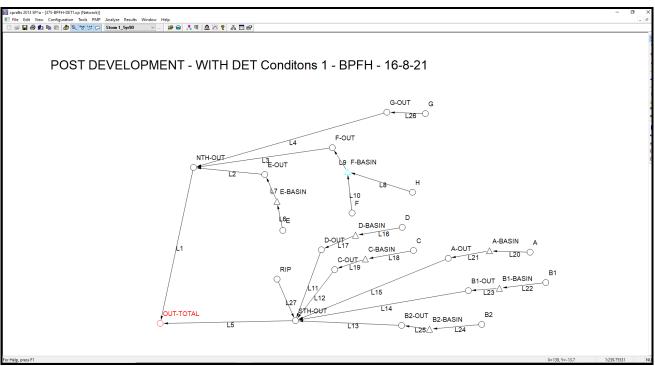


Diagram 6 – Post Development (*With Detention*) RAFTS Network

Detention is proposed to be implemented within the proposed future subdivision/development in the form of large above ground community basins. Seven basins are proposed with a total volume of **36,150m<sup>3</sup>** (*refer to the Figures section at the rear of this report*).

The overall objective of the modelling was to ensure that post development flows did not increase beyond pre development levels. In RAFTS this was modelled by introduction of a detention node incorporating multiple outlet controls and high early discharge. The OSD outlet configuration was determined through iteration to optimise the storage volumes required for the critical storms only.

 Table 12 provides a summary of the resultant SSR and PSD values.

	5yr	ARI	20yr	ARI	100y	r ARI	Мах	OK	
Basin	PSD	V used	PSD	V used	PSD	V used	Basin V	Y or N	
F	0.29	9,468	1.22	11,096	2.40	13,177	13,150	Yes	
E	0.09	2,891	0.37	3,462	0.82	3,916	4,000	Yes	
D	0.07	1,299	0.23	1,653	0.44	2,086	2,200	Yes	
С	0.06	1,761	0.21	2,440	0.48	3,119	3,200	Yes	
А	0.26	5,873	0.81	7,288	1.60	9,014	9,100	Yes	
B1	0.05	1,027	0.15	1,460	0.34	1,865	2,000	Yes	
B2	0.05	1,117	0.14	1,709	0.30	2,352	2,500	Yes	
TOTAL		23,436		29,108		35,529	36,150		

Table 12 – Post Development (*With Detention*) PSD and Det. Volume Results



The results of the post development condition RAFTS model are summarised in **Table 13**. Details of the model are included at **Appendix C.** 

The RAFTS results show that under post development conditions, the combined total peak flows generated by the site in the 100 year ARI event increases from **9.20 m<sup>3</sup>/s** to **29.00 m<sup>3</sup>/s** and then back to **7.71m<sup>3</sup>/s** after detention. An illustration of the critical 100yr ARI post development condition site hydrograph is included at **Diagram 7**.

The detention volume required to achieve this compliant result was greatest in the 100yr ARI event at **35,529m<sup>3</sup>** (254m<sup>3</sup>/ha for the 140ha Dev A).

The peak flows in both the 20yrARI, 5yr and 2yr ARI events were also shown to be reduced below pre development conditions.

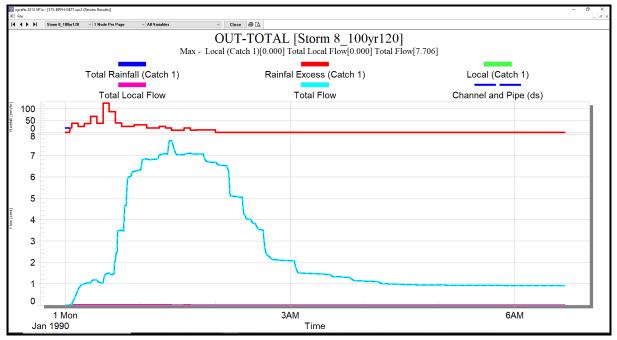


Diagram 7 – Post Development 100yr ARI Hydrograph



Storm Frequency	Storm Duration (min)	G- OUT	F- OUT	E- OUT	NTH- OUT	D- OUT	C- OUT	A- OUT	B1- OUT	B2- OUT	RIP	STH- OUT	OUT- TOTAL
100yr ARI	45	0.47	2.40	0.82	3.45	0.44	0.48	1.60	0.34	0.30	0.37	3.36	6.69
100yr ARI	60	0.51	2.81	0.82	3.74	0.44	0.48	1.60	0.34	0.30	0.50	3.65	7.22
100yr ARI	90	0.52	2.77	0.82	3.70	0.44	0.48	1.60	0.34	0.30	0.60	3.76	7.41
100yr ARI	120	0.47	3.04	0.82	3.96	0.44	0.48	1.60	0.34	0.30	0.63	3.79	7.71
100yr ARI	180	0.26	2.40	0.82	3.39	0.44	0.48	1.60	0.34	0.30	0.64	3.80	7.10
20yr ARI	90	0.40	1.22	0.37	1.71	0.23	0.21	0.81	0.15	0.14	0.25	1.79	3.44
20yr ARI	120	0.36	1.67	0.37	2.10	0.23	0.28	0.81	0.15	0.14	0.30	1.91	3.89
5yr ARI	90	0.29	0.29	0.09	0.67	0.07	0.06	0.26	0.05	0.05	0.08	0.57	1.23
5yr ARI	120	0.27	0.58	0.09	0.68	0.07	0.08	0.31	0.05	0.05	0.12	0.66	1.32

Note 1. Values shown in bold are critical.

2. Model 375BPFH-Det1.xp



## 6. STORMWATER DRAINAGE

A road based stormwater drainage system is proposed in the local streets surrounding the proposed future subdivision. The proposed minor/pipe system capacity would be designed with a minimum 10yr ARI capacity in accordance with the City of Wagga Wagga Council standards. Major flows up to and including the 100yr ARI would be carried safely overland within the road reserve (DV<0.4).

All proposed future subdivision drainage will discharge into the proposed basins for treatment/detention.

Outflow from the basins will be discharged to the receiving waters via a combination of stabilised low flow outlets and high flow weirs.

#### 7. MAINSTREAM FLOODING

Existing Mainstream 100yr ARI flood levels have been provided for the Northern parts of the proposed development adjacent to Marshalls Creek (*approximately RL.183.15*).

All residential development in the northern catchment will be sited a minimum of 500mm above this 100yr ARI flood level.

Existing flooding extents for the southern catchment (*i.e., generated by the Gregadoo Creek and Airport external catchments*) are unknown. A flood study will be completed at DA stage to determine these details and inform the future subdivision design.

## 8. RIPARIAN CORRIDOR WORKS

The southern area of the proposed development will be split by two open space corridors that will convey/contain floodwaters from upstream catchments and provide a valuable riparian function.

The corridors will contain a meandering low flow channel that is rock lined and incorporate a series of natural pools and riffles. Overbanks will be wide and flat to maximise flood conveyance and be planted with native vegetation. The edges of the corridors will likely contain a vertical component (*i.e., rock walls*) to ensure the future subdivision sits a minimum of 500mm above the 100yr flood level within the floodway.

The proposed bio-retention/detention basins will sit within the corridors with batters/walls to separate local runoff from upstream regional catchment flows. Timing between local and regional catchments will also play a role.

A 100m wide corridor will run from Inglewood Road to the western boundary of the site along the approximate existing alignment of Gregadoo Creek. At Inglewood Road this corridor will convey flows from a large upstream catchment of approximately 2,500ha.

A 60m wide corridor will run from Elizabeth Avenue running west to join the proposed 100m wide corridor. This will convey flows from the approximate 50ha upstream airport catchment.



#### 8. CONCLUSIONS

A conceptual stormwater management plan has been formulated to support the planning proposal for the proposed 500 lot future subdivision of Brunslea Park, Forest Hill. The strategy ensures that both the water quality and quantity treatment targets of the City of wagga Wagga Council are not only achieved but exceeded.

#### 9. QUALIFIERS

This report has been prepared by Mr Michael Shaw. A copy of Michael's CV is included at **Appendix D**.

This report has been prepared for the benefit of Yarraman Developments and the City of Wagga Wagga Council with relation to the particular project described and no responsibility is accepted for the use of any part of this report in any other context or for any other purpose. Copyright in this report is the property of Civil Certification. In preparing this report I have used a degree of care, skill and diligence normally exercised by consulting engineers in similar circumstances and locality. No other warranty expressed or implied is made or intended.

We trust this report is satisfactory. Should you have any further queries, please do not hesitate to contact me on 0412 264 237.

Yours faithfully

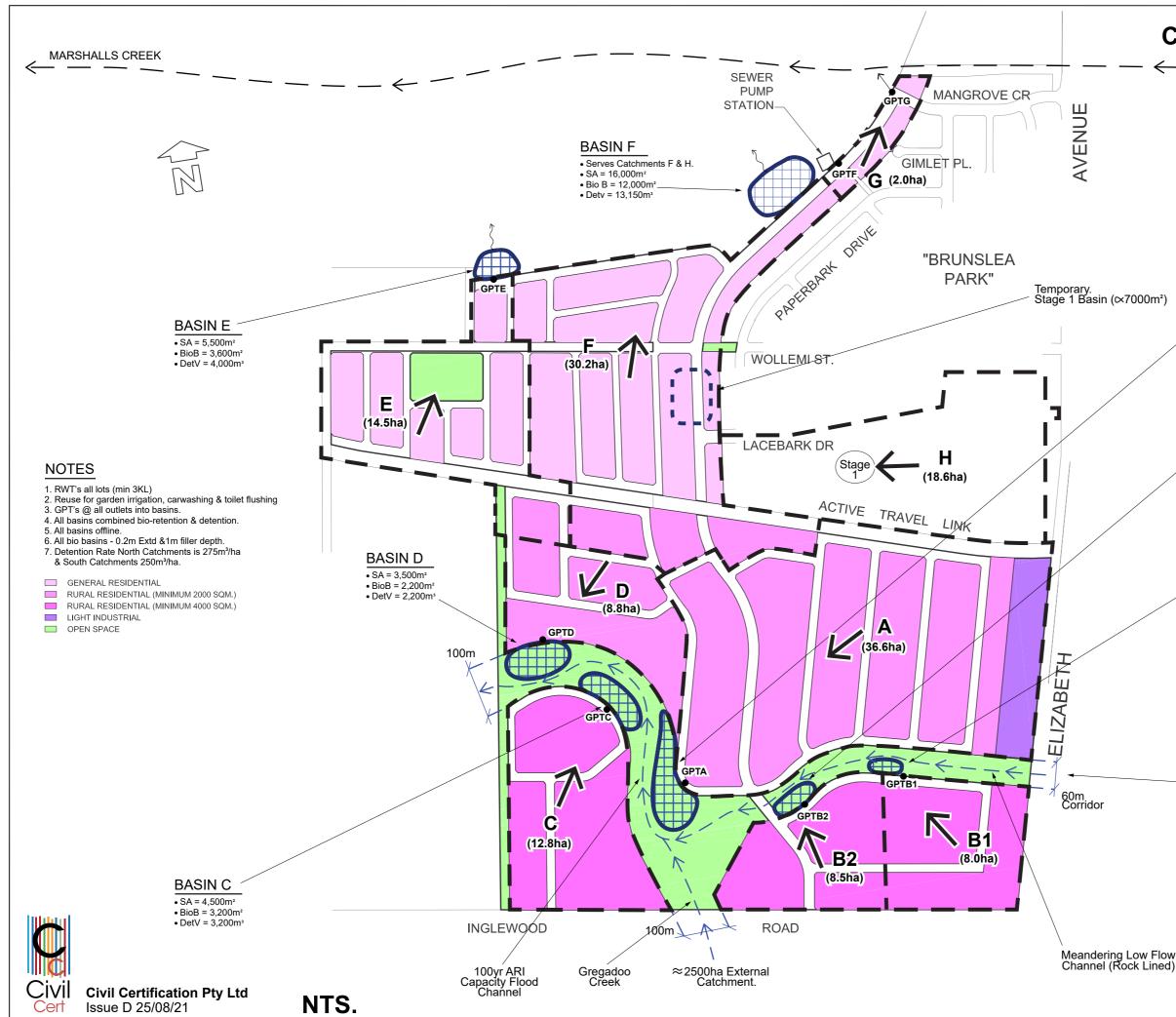
#### CIVIL CERTIFICATION PTY LTD

Michael Shaw BE(Civil), MIEAust, CPEng, NER(Civil), APEC Engineer IntPE(Aus) Accredited Certifier (BDC 0816), RPEQ (24197)

Director 0412 264 237 mshaw@civilvcert.com



# FIGURES



# **CONCEPT STORMWATER MANAGEMENT PLAN**

/	BASIN A							
	• SA = 12,000m							

• BioB = 9,100m<sup>2</sup> • DetV = 9,100m<sup>3</sup>

#### **BASIN B2** • SA = 3,500m<sup>2</sup>

• BioB = 2,500m<sup>2</sup> DetV = 2.500m<sup>3</sup>

#### BASIN B1 • SA = 2,500m<sup>2</sup> • BioB = 2,000m<sup>2</sup>

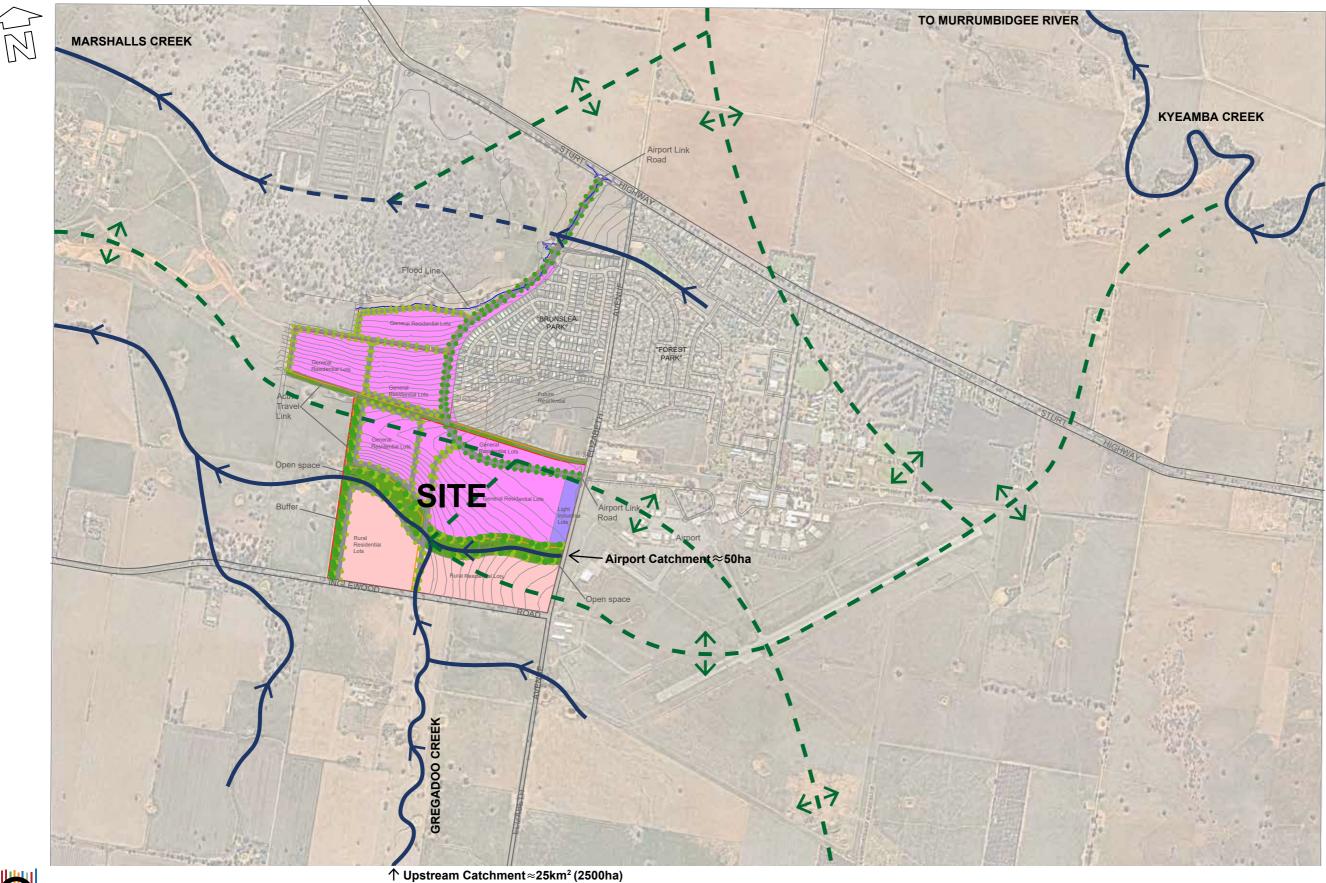
• DetV = 2,000m<sup>3</sup>

.≈50ha External Catchment.





# **REGIONAL CATCHMENT PLAN**





Civil Certification Pty Ltd Issue A 25/08/21

NTS.



Catchment Boundary

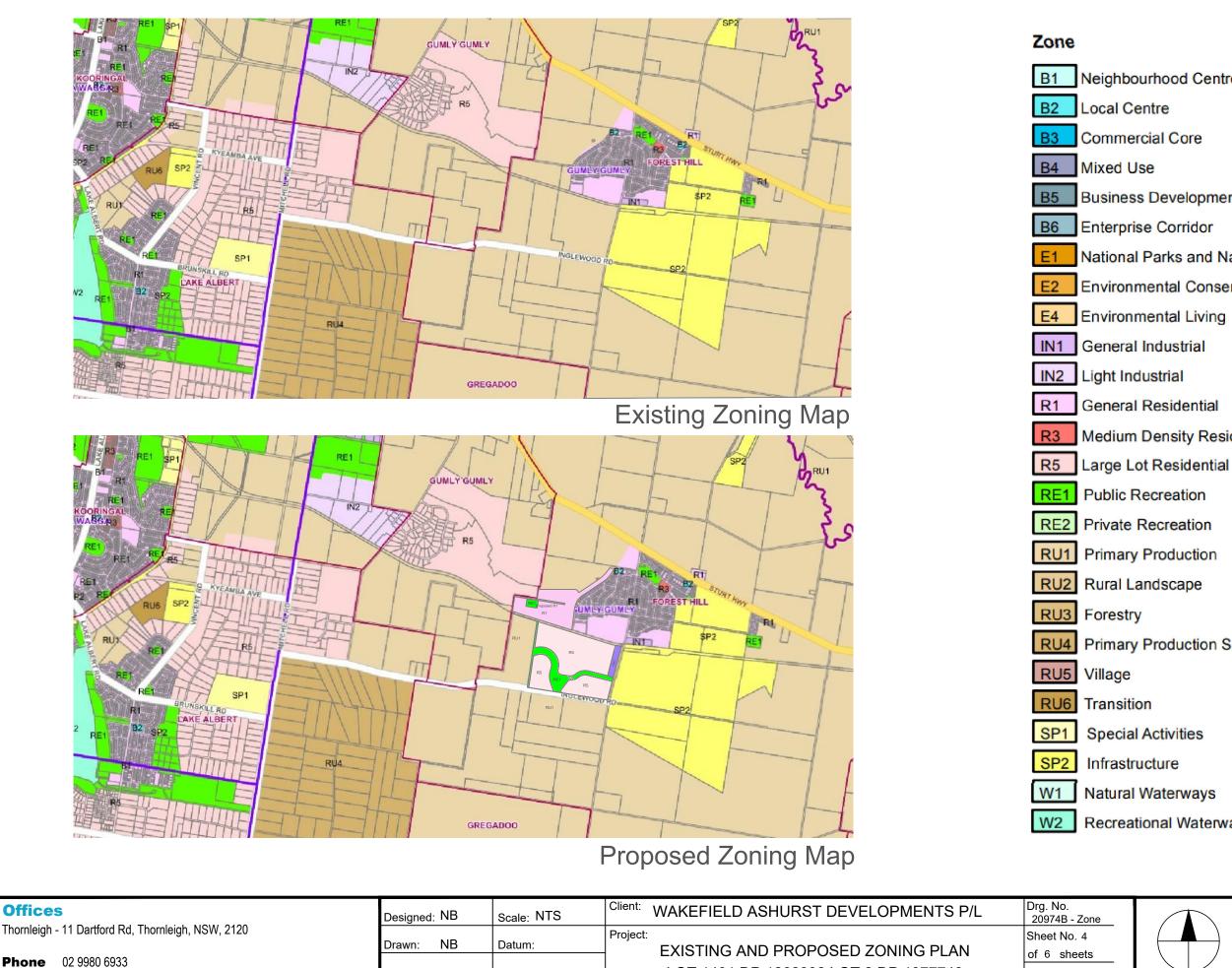
Watercourse

Fall Direction





# APPENDIX A (Proposed Development Layout & Zoning Map)



Checked:

L.G.A: WAGGA WAGGA

Email

Web

dfp@dfpplanning.com.au

www.dfpplanning.com.au

Date: 19/10/21

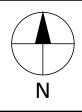
A3

LOT 1401 DP 1262802 LOT 2 DP 1077748

LOT 2 DP 1262040 LOT 1 DP 1220149

- Neighbourhood Centre
- **Commercial Core**
- Business Development
- Enterprise Corridor
- National Parks and Nature Reserves
- Environmental Conservation
- **General Residential**
- Medium Density Residential
- RU4 Primary Production Small Lots

  - Natural Waterways
  - **Recreational Waterways**



Our Ref:

20974B

Postal PO BOX 230 Pennant Hills N.S.W. 1715



planning consultants



# **APPENDIX B (MUSIC)**



#### Post Dev (with Treatment) MUSIC Model

#### Source nodes

Location, total-site-pre-ag, rip-cor-sth, a, b1, b2, c, d, e, h, f, g ID,1,6,7,8,9,10,11,17,19,20,23 Node Type, AgriculturalSourceNode, ForestSourceNode, UrbanSourceNode, UrbanSour eNode, UrbanSourceNode, UrbanSourceNode, UrbanSourceNode, UrbanSourceNode, UrbanSourceNode Zoning Surface Type,,,Mixed,Mixed,Mixed,Mixed,Mixed,Mixed,Mixed,Mixed Total Area (ha),155.7,15.7,36.6,8,8.5,12.8,8.8,14.5,18.6,30.2,2 Area Impervious (ha),2.74217910447762,0.276507462686566,18.3,3.21611940298507,3.41712686567164,5.14579104477612,4.4,9.3 8117537313433,12.1038805970149,19.538723880597,1.2939552238806 Area Pervious (ha),152.957820895522,15.4234925373134,18.3,4.78388059701493,5.08287313432836,7.65420895522388,4.4,5.11 882462686567,6.49611940298508,10.661276119403,0.706044776119403 Groundwater Initial Depth (mm),1,10,1,1,1,1,1,1,1,1,1,1 Groundwater Daily Baseflow Rate (%),1,5,1,1,1,1,1,1,1,1,1,1 Groundwater Daily Deep Seepage Rate (%),15,0,15,15,15,15,15,15,15,15,15 Stormflow Total Suspended Solids Mean (log mg/L), 2.041, 1.9, 2.19 Stormflow Total Suspended Solids Standard Deviation (log Stormflow Total Suspended Solids Estimation Method, Stochastic, Stochastic ic, Stochastic, Stochastic Stormflow Total Suspended Solids Serial Correlation,0,0,0,0,0,0,0,0,0,0,0 Stormflow Total Phosphorus Mean (log mg/L),-0.662,-1.1,-0.65 0.65, -0.65Stormflow Total Phosphorus Standard Deviation (log Stormflow Total Phosphorus Estimation Method, Stochastic, Stochastic ic, Stochastic, Stochastic Stormflow Total Phosphorus Serial Correlation,0,0,0,0,0,0,0,0,0,0,0,0 Stormflow Total Nitrogen Mean (log mg/L),0.314, 0.075, 0.427, 0.427, 0.427, 0.427, 0.427, 0.427, 0.427, 0.427, 0.427 Stormflow Total Nitrogen Standard Deviation (log Stormflow Total Nitrogen Estimation Method, Stochastic, Stochastic ic, Stochastic, Stochastic Baseflow Total Suspended Solids Standard Deviation (log Baseflow Total Suspended Solids Estimation Method, Stochastic, Stochastic ic, Stochastic, Stochastic Baseflow Total Suspended Solids Serial Correlation,0,0,0,0,0,0,0,0,0,0,0 Baseflow Total Phosphorus Mean (log mg/L), -0.88, -1.5, -0.82, -0 0.82 Baseflow Total Phosphorus Standard Deviation (log Baseflow Total Phosphorus Estimation Method, Stochastic, Stochastic ic, Stochastic, Stochastic Baseflow Total Phosphorus Serial Correlation,0,0,0,0,0,0,0,0,0,0,0 Baseflow Total Nitrogen Standard Deviation (log Baseflow Total Nitrogen Estimation Method, Stochastic, Stochastic ic, Stochastic, Stochastic Baseflow Total Nitrogen Serial Correlation,0,0,0,0,0,0,0,0,0,0,0,0 



Flow based constituent generation - pervious flow column, , , , , , , , , , , Flow based constituent generation - impervious flow column, , , , , , , , , , , Flow based constituent generation - unit, , , , OUT - Mean Annual Flow (ML/yr),149,7.41,135,25.0,26.6,40.0,32.5,66.0,84.7,138,9.11 OUT - TSS Mean Annual Load (kg/yr),16.1E3,308,27.4E3,5.05E3,5.18E3,8.05E3,6.35E3,13.3E3,17.0E3,27.3E3,1.83E3 OUT - TP Mean Annual Load (kg/yr),32.2,0.364,35.7,6.63,6.79,10.9,8.34,18.0,23.1,37.6,2.48 OUT - TN Mean Annual Load (kg/yr),304,7.39,393,72.7,79.5,116,97.4,191,245,395,26.4 OUT - Gross Pollutant Mean Annual Load (kg/yr),160,13.6,5.33E3,997,1.06E3,1.59E3,1.28E3,2.50E3,3.20E3,5.20E3,344 Rain In (ML/yr),1020.26,102.877,239.829,52.4216,55.6979,83.8746,57.6637,95.0142,121.88,197.892,13.1054 ET Loss (ML/yr),815.973,96.0439,97.8625,25.6688,27.2731,41.0698,23.5297,27.1392,34.8132,56.5245,3.74336 Deep Seepage Loss (ML/yr), 58.209, 0, 6.98115, 1.83112, 1.94556, 2.92979, 1.67853, 1.93603, 2.48346, 4.03228, 0.267039 Baseflow Out (ML/yr),3.29852,4.9134,0.395599,0.103764,0.110249,0.166022,0.0951166,0.109708,0.140729,0.228496,0.01513 2.2 Imp. Stormflow Out (ML/yr), 20.4051, 1.77459, 119.914, 20.9686, 22.2792, 33.5498, 28.8319, 61.7592, 79.2222, 128.63, 8.51852 Perv. Stormflow Out (ML/yr),125.238,0.72424,15.0201,3.9397,4.18593,6.30352,3.61139,4.16541,5.34322,8.67554,0.574539 Total Stormflow Out (ML/yr),145.643,2.49883,134.935,24.9083,26.4651,39.8534,32.4433,65.9246,84.5654,137.305,9.09306 Total Outflow (ML/yr),148.942,7.41223,135.33,25.0121,26.5754,40.0194,32.5384,66.0343,84.7061,137.534,9.10819 Change in Soil Storage (ML/yr),-2.86991,-0.578498,-0.344195,-0.0902807,-0.0959232,-0.144449,-0.0827573,-0.095453,-0.122443,-0.198806,-0.0131659 TSS Baseflow Out (kg/yr),82.855,41.2839,10.4404,2.73847,2.96733,4.58584,2.56005,3.03036,3.88722,6.12163,0.417981 TSS Total Stormflow Out (kg/yr),16006.3,267.192,27389.8,5050.13,5177.44,8040.48,6349.97,13251,16997.9,27303.2,1827.72 TSS Total Outflow (kg/yr),16089.1,308.476,27400.3,5052.87,5180.41,8045.07,6352.53,13254,17001.7,27309.3,1828.14 TP Baseflow Out (kg/yr),0.434829,0.161692,0.0657557,0.0172474,0.0186503,0.0271546,0.0160905,0.017944,0.0230178,0.038043 3,0.002475 TP Total Stormflow Out (kg/yr), 31.7169, 0.202529, 35.6798, 6.60856, 6.77318, 10.8619, 8.32036, 17.9936, 23.0814, 37.5249, 2.48187 TP Total Outflow (kg/yr), 32.1517, 0.364221, 35.7456, 6.62581, 6.79183, 10.889, 8.33645, 18.0115, 23.1044, 37.563, 2.48435 TN Baseflow Out (kg/yr),3.91127,3.82374,0.850392,0.223054,0.242238,0.364266,0.208989,0.24071,0.308772,0.489421,0.033201 TN Total Stormflow Out (kg/yr),300.117,3.56822,391.73,72.4526,79.2807,115.583,97.2178,190.809,244.762,394.92,26.3185 TN Total Outflow (kg/yr), 304.028, 7.39196, 392.581, 72.6756, 79.5229, 115.947, 97.4268, 191.05, 245.071, 395.409, 26.3517 GP Total Outflow (kg/yr),160.039,14.9775,5337.72,998.459,1060.86,1597.53,1283.39,2497.32,3203.46,5201.32,344.458

#### No Imported Data Source nodes

#### USTM treatment nodes

Location, basin-a, basin-b1, basin-b2, basin-c, basin-d, basin-e, basin-f ID, 12, 13, 14, 15, 16, 18, 22 Node Type, BioRetentionNodeV4, BioRetentionNodeV4, BioRetentionNodeV4, BioRetentionNodeV4, BioRetentionNodeV4, Bio RetentionNodeV4, BioRetentionNodeV4 Lo-flow bypass rate (cum/sec),0,0,0,0,0,0,0 Hi-flow bypass rate (cum/sec),10,10,10,10,10,10,10 Inlet pond volume, Area (sqm),9100,2000,2500,3200,2200,3600,12000 Initial Volume (m^3), , , , , , , , , Extended detention depth (m),0.2,0.2,0.2,0.2,0.2,0.2,0.2 Number of Rainwater tanks, , Permanent Pool Volume (cubic metres), , , , , , Proportion vegetated, , , , , , , Equivalent Pipe Diameter (mm), , , Overflow weir width (m),25,20,20,25,25,25,50 Notional Detention Time (hrs), , , , , , , Orifice Discharge Coefficient, , , Number of CSTR Cells, 3, 3, 3, 3, 3, 3, 3, 3 Total Suspended Solids - k (m/yr),8000,8000,8000,8000,8000,8000,8000

```
Civil
Cert
```

```
Total Suspended Solids - C* (mg/L),20,20,20,20,20,20,20 Total Suspended Solids - C** (mg/L), , , , , , ,
Total Phosphorus - k (m/yr),6000,6000,6000,6000,6000,6000
Total Phosphorus - C* (mg/L),0.13,0.13,0.13,0.13,0.13,0.13,0.13
Total Phosphorus - C** (mg/L), , , , , , ,
Total Nitrogen - k (m/yr),500,500,500,500,500,500,500
Total Nitrogen - C* (mg/L), 1.4, 1.4, 1.4, 1.4, 1.4, 1.4, 1.4
Total Nitrogen - C** (mg/L), , , , , , ,
Threshold Hydraulic Loading for C** (m/yr), , , , , ,
Horizontal Flow Coefficient, 3, 3, 3, 3, 3, 3, 3, 3
Reuse Enabled, Off, Off, Off, Off, Off, Off, Off
Max drawdown height (m), , , , , , , , , Annual Demand Enabled,Off,Off,Off,Off,Off,Off,Off
Annual Demand Value (ML/year), , , , , , ,
Annual Demand Distribution, , , , , , ,
Annual Demand Monthly Distribution: Jan, , , , ,
Annual Demand Monthly Distribution: Feb, , , , , , , Annual Demand Monthly Distribution: Mar, , , , , , ,
Annual Demand Monthly Distribution: Apr, , , , , ,
Annual Demand Monthly Distribution: May, , , ,
Annual Demand Monthly Distribution: Jun, , , ,
Annual Demand Monthly Distribution: Jul, , , , ,
Annual Demand Monthly Distribution: Aug, , , , ,
Annual Demand Monthly Distribution: Sep, , , , ,
Annual Demand Monthly Distribution: Oct, , , ,
Annual Demand Monthly Distribution: Nov, , , , , , ,
Annual Demand Monthly Distribution: Dec, ,
Daily Demand Enabled, Off, Off, Off, Off, Off, Off, Off
Daily Demand Value (ML/day), , ,
Custom Demand Enabled, Off, Off, Off, Off, Off, Off
Custom Demand Time Series File, , , , , ,
Custom Demand Time Series Units, ,
Filter area (sqm),4550,1000,1250,1600,1100,1800,6000
Filter perimeter (m),270,126,141,160,132,170,309
Filter depth (m),1,1,1,1,1,1,1
Filter Median Particle Diameter (mm), , ,
Infiltration Media Porosity, 0.35, 0.35, 0.35, 0.35, 0.35, 0.35, 0.35
Length (m), , , , , , ,
Bed slope, , , , , , , , Base Width (m), , , , , , ,
Top width (m), , , , , , , , Vegetation height (m), , ,
Vegetation Type, Vegetated with Effective Nutrient Removal Plants, Vegetated with Effective Nutrient
Removal Plants, Vegetated with Effective Nutrient Removal Plants, Vegetated with Effective Nutrient
Removal Plants, Vegetated with Effective Nutrient Removal Plants, Vegetated with Effective Nutrient
Removal Plants, Vegetated with Effective Nutrient Removal Plants
Total Nitrogen Content in Filter (mg/kg),800,800,800,800,800,800,800
Is Base Lined?, No, No, No, No, No, No, No, No, No
Is Underdrain Present?, Yes, Yes, Yes, Yes, Yes, Yes, Yes
Is Submerged Zone Present?, No, No, No, No, No, No, No, No
Submerged Zone Depth (m), , ,
B for Media Soil Texture, 13, 13, 13, 13, 13, 13, 13, 13
Proportion of upstream impervious area treated, , ,
Exfiltration Rate (mm/hr),0.4,0.4,0.4,0.4,0.4,0.4,0.4
Evaporative Loss as % of PET,100,100,100,100,100,100
Depth in metres below the drain pipe, , , , , ,
TSS A Coefficient, , , , , , ,
TSS B Coefficient, , , , , , ,
TP A Coefficient, , , , , , ,
TP B Coefficient, , , , , , ,
TN A Coefficient, , , , , , ,
TN B Coefficient, ,
Sfc, 0.61, 0.61, 0.61, 0.61, 0.61, 0.61, 0.61
s*,0.37,0.37,0.37,0.37,0.37,0.37,0.37
Sw, 0.11, 0.11, 0.11, 0.11, 0.11, 0.11, 0.11
Sh,0.05,0.05,0.05,0.05,0.05,0.05,0.05
Emax (m/day),0.008,0.008,0.008,0.008,0.008,0.008,0.008
Ew (m/day),0.001,0.001,0.001,0.001,0.001,0.001,0.001
IN - Mean Annual Flow (ML/yr),135,25.0,26.6,40.0,32.5,66.0,222
IN - TSS Mean Annual Load (kg/yr),8.22E3,1.52E3,1.55E3,2.41E3,1.91E3,3.98E3,13.3E3
IN - TP Mean Annual Load (kg/yr), 25.0, 4.64, 4.75, 7.62, 5.84, 12.6, 42.5
IN - TN Mean Annual Load (kg/yr),393,72.7,79.5,116,97.4,191,640
IN - Gross Pollutant Mean Annual Load (kg/yr),107,19.9,21.2,31.9,25.6,49.9,168
```



OUT - Mean Annual Flow (ML/yr),117,21.1,21.8,33.8,28.2,58.7,198 OUT - TSS Mean Annual Load (kg/yr), 305, 55.6, 50.7, 89.2, 73.6, 155, 524 OUT - TP Mean Annual Load (kg/yr),14.0,2.52,2.60,4.03,3.35,6.97,23.5 OUT - TN Mean Annual Load (kg/yr),77.0,13.9,13.8,22.2,18.6,38.4,129 OUT - Gross Pollutant Mean Annual Load (kg/yr),0.00,0.00,0.00,0.00,0.00,0.00,0.00 Flow In (ML/yr),135.33,25.012,26.5753,40.0193,32.5383,66.0342,222.239 ET Loss (ML/yr),10.2473,2.23805,2.78588,3.58088,2.47702,4.08541,13.622 Infiltration Loss (ML/yr),7.77051,1.6348,1.96615,2.61542,1.87688,3.21737,10.742 Low Flow Bypass Out (ML/yr),0,0,0,0,0,0,0 High Flow Bypass Out (ML/yr),0,0,0,0,0,0,0 Orifice / Filter Out (ML/yr),113.49,20.4397,21.4936,32.708,27.2573,56.6662,190.781 Weir Out (ML/yr), 3.81894, 0.700306, 0.331219, 1.11622, 0.926429, 2.05795, 7.06857 Transfer Function Out (ML/yr), 0, 0, 0, 0, 0, 0, 0Reuse Supplied (ML/yr),0,0,0,0,0,0,0 Reuse Requested (ML/yr),0,0,0,0,0,0,0 % Reuse Demand Met, 0, 0, 0, 0, 0, 0, 0 % Load Reduction, 13.3164, 15.4809, 17.8756, 15.4804, 13.3829, 11.0701, 10.9744 TSS Flow In (kg/yr),8220.06,1515.86,1554.12,2413.51,1905.76,3976.2,13293.3 TSS ET Loss (kg/yr),0,0,0,0,0,0,0 TSS Infiltration Loss (kg/yr), 17.7528, 3.78165, 4.49926, 6.03801, 4.29444, 7.30458, 24.5098 TSS Low Flow Bypass Out (kg/yr), 0, 0, 0, 0, 0, 0, 0TSS High Flow Bypass Out (kg/yr), 0, 0, 0, 0, 0, 0, 0TSS Orifice / Filter Out (kg/yr),227.185,41.3233,43.9189,66.1263,54.5833,112.16,377.395 TSS Weir Out (kg/yr),78.2005,14.2867,6.74359,23.0968,19.0261,42.4589,146.647 TSS Transfer Function Out (kg/yr),0,0,0,0,0,0,0 TSS Reuse Supplied (kg/yr),0,0,0,0,0,0,0 TSS Reuse Requested (kg/yr),0,0,0,0,0,0,0 TSS % Reuse Demand Met,0,0,0,0,0,0,0 TSS % Load Reduction, 96.2849, 96.3314, 96.7401, 96.3032, 96.1375, 96.1114, 96.0578 TP Flow In (kg/yr),25.0219,4.63806,4.75427,7.62229,5.8355,12.608,42.467 TP ET Loss (kg/yr),0,0,0,0,0,0,0 TP Infiltration Loss (kg/yr),0.931631,0.19648,0.236585,0.314277,0.225007,0.384421,1.2841 TP Low Flow Bypass Out (kg/yr),0,0,0,0,0,0,0 TP High Flow Bypass Out (kg/yr),0,0,0,0,0,0,0 TP Orifice / Filter Out (kg/yr),13.4577,2.42936,2.56103,3.88751,3.23247,6.70197,22.5609 TP Weir Out (kg/yr),0.504054,0.0921842,0.0431235,0.146034,0.1209,0.26879,0.928529 TP Transfer Function Out (kg/yr),0,0,0,0,0,0,0 TP Reuse Supplied (kg/yr),0,0,0,0,0,0,0 TP Reuse Requested (kg/yr),0,0,0,0,0,0,0 TP % Reuse Demand Met, 0, 0, 0, 0, 0, 0, 0 TP % Load Reduction,44.2016,45.6336,45.2249,47.0822,42.535,44.7118,44.6878 TN Flow In (kg/yr),392.58,72.6754,79.5227,115.947,97.4265,191.049,640.479 TN ET Loss (kg/yr),0,0,0,0,0,0,0 TN Infiltration Loss (kg/yr),4.81092,1.01313,1.20869,1.61606,1.16071,1.9893,6.64284 TN Low Flow Bypass Out (kg/yr),0,0,0,0,0,0,0 TN High Flow Bypass Out (kg/yr),0,0,0,0,0,0,0 TN Orifice / Filter Out (kg/yr),68.327,12.3346,13.0204,19.7559,16.4497,34.0734,114.509 TN Weir Out (kg/yr),8.69366,1.57217,0.783476,2.44214,2.15743,4.37176,14.4919 TN Transfer Function Out (kg/yr),0,0,0,0,0,0,0 TN Reuse Supplied (kg/yr),0,0,0,0,0,0,0 TN Reuse Requested (kg/yr),0,0,0,0,0,0,0 TN % Reuse Demand Met,0,0,0,0,0,0,0 TN % Load Reduction,80.3809,80.8645,82.6416,80.8549,80.9013,79.8768,79.8587 GP Flow In (kg/yr),106.624,19.937,21.183,31.8992,25.6363,49.9089,167.969 GP ET Loss (kg/yr),0,0,0,0,0,0,0 GP Infiltration Loss (kg/yr),0,0,0,0,0,0,0 GP Low Flow Bypass Out (kg/yr), 0, 0, 0, 0, 0, 0, 0GP High Flow Bypass Out (kg/yr),0,0,0,0,0,0,0 GP Orifice / Filter Out (kg/yr),0,0,0,0,0,0,0 GP Weir Out (kg/yr),0,0,0,0,0,0,0 GP Transfer Function Out (kg/yr),0,0,0,0,0,0,0 GP Reuse Supplied (kg/yr),0,0,0,0,0,0,0 GP Reuse Requested (kg/yr),0,0,0,0,0,0,0 GP % Reuse Demand Met, 0, 0, 0, 0, 0, 0 GP % Load Reduction, 100, 100, 100, 100, 100, 100, 100 

#### Generic treatment nodes

Location,gpt-f (CDS2028),gpt-e (CDS1512),gpt-g (CDS1009),gpt-a (CDS2018),gpt-b1 ( CDS1015), gpt-b2 (CDS1015),gpt-c (CDS1512),gpt-d (CDS1015) ID,21,24,25,26,27,28,29,30 Node Type,GPTNode,GPTNode,GPTNode,GPTNode,GPTNode,GPTNode,GPTNode Lo-flow bypass rate (cum/sec),0,0,0,0,0,0,0 Hi-flow bypass rate (cum/sec),0.8,0.22,0.1,0.53,0.18,0.18,0.22,0.18



Flow Transfer Function Input (cum/sec),0,0,0,0,0,0,0,0 Output (cum/sec),0,0,0,0,0,0,0,0 Input (cum/sec),10,10,10,10,10,10,10,10 Output (cum/sec),10,10,10,10,10,10,10,10 Input (cum/sec), , , , , , , , Input (cum/sec), , , , , , , , Output (cum/sec), , , , , , , , Input (cum/sec), , , , , , , , Output (cum/sec), , , , , , , Gross Pollutant Transfer Function Enabled, True, True, True, True, True, True, True, True Input (kg/ML),0,0,0,0,0,0,0,0 Output (kg/ML),0,0,0,0,0,0,0,0 Input (kg/ML),100,100,100,100,100,100,100 Output (kg/ML),2,2,2,2,2,2,2,2 Output (kg/ML), , , , , , , , Input (kg/ML), , , , , , , , Output (kg/ML), , , , , , , , , Input (kg/ML), , , , , , , , , Total Nitrogen Transfer Function Enabled, True, True, True, True, True, True, True, True Input (mg/L),0,0,0,0,0,0,0,0 Output (mg/L),0,0,0,0,0,0,0,0 Input (mg/L), 50, 50, 50, 50, 50, 50, 50, 50 Output (mg/L), 50, 50, 50, 50, 50, 50, 50, 50 Input (mg/L), , , , , , , , Output (mg/L), , , , , , , , , Input (mg/L), , , , , , , , , Output (mg/L), , , , , , , , Input (mg/L), , , , , , , , , Enabled, True, True, True, True, True, True, True, True Input (mg/L),0,0,0,0,0,0,0,0 Output (mg/L),0,0,0,0,0,0,0,0 Input (mg/L),100,100,100,100,100,100,100,100 Output (mg/L),70,70,70,70,70,70,70,70 Input (mg/L), , , , , , , , , Output (mg/L), , , , , , , , ,



Input (mg/L), , , , , , , , , Output (mg/L), , , , , , , , Input (mg/L), , , , , , , , Output (mg/L), , , , , , , , Input (mg/L), , , , , , , , Output (mg/L), , , , , , , , , Enabled, True, True, True, True, True, True, True, True Input (mg/L),0,0,0,0,0,0,0,0 Output (mg/L),0,0,0,0,0,0,0,0 Input (mg/L),100,100,100,100,100,100,100,100 Output (mg/L), 30, 30, 30, 30, 30, 30, 30, 30, 30 Input (mg/L), , , , , , , , Output (mg/L), , , , , , , , , Input (mg/L), , , , , , , , Output (mg/L), , , , , , , , Input (mg/L), , , , , , , , Output (mg/L), , , , , , , , , Input (mg/L), , , , , , , , , Output (mg/L), , , , , , , , Input (mg/L), , , , , , , , Output (mg/L), , , , , , , , Input (mg/L), , , , , , , , Output (mg/L), , , , , , , , , Input (mg/L), , , , , , , , Output (mg/L), , , , , TSS Flow based Efficiency, , , , , TP Flow based Efficiency, , , , , , TN Flow based Efficiency, , , , , , , GP Flow based Efficiency, , , , IN - Mean Annual Flow (ML/yr),222,66.0,9.11,135,25.0,26.6,40.0,32.5 IN - TSS Mean Annual Load (kg/yr),44.3E3,13.3E3,1.83E3,27.4E3,5.05E3,5.18E3,8.05E3,6.35E3 IN - TP Mean Annual Load (kg/yr),60.7,18.0,2.48,35.7,6.63,6.79,10.9,8.34 IN - TN Mean Annual Load (kg/yr),640,191,26.4,393,72.7,79.5,116,97.4 IN - Gross Pollutant Mean Annual Load (kg/yr), 8.40E3, 2.50E3, 344, 5.33E3, 997, 1.06E3, 1.59E3, 1.28E3 OUT - Mean Annual Flow (ML/yr),222,66.0,9.11,135,25.0,26.6,40.0,32.5 OUT - TSS Mean Annual Load (kg/yr),13.3E3,3.98E3,548,8.22E3,1.52E3,1.55E3,2.41E3,1.91E3 OUT - TP Mean Annual Load (kg/yr),42.5,12.6,1.74,25.0,4.64,4.75,7.62,5.84 OUT - TN Mean Annual Load (kg/yr),640,191,26.4,393,72.7,79.5,116,97.4 OUT - Gross Pollutant Mean Annual Load (kg/yr),168,49.9,6.88,107,19.9,21.2,31.9,25.6 Flow In (ML/yr),222.24,66.0342,9.10815,135.33,25.0121,26.5754,40.0193,32.5383 ET Loss (ML/yr),0,0,0,0,0,0,0,0 Infiltration Loss (ML/yr),0,0,0,0,0,0,0,0 Low Flow Bypass Out (ML/yr),0,0,0,0,0,0,0,0 High Flow Bypass Out (ML/yr),0,0,0,0,0,0,0,0 Orifice / Filter Out (ML/yr),0,0,0,0,0,0,0,0 Weir Out (ML/yr),0,0,0,0,0,0,0,0 Transfer Function Out (ML/yr),222.24,66.0342,9.10815,135.33,25.0121,26.5754,40.0193,32.5383 Reuse Supplied (ML/yr),0,0,0,0,0,0,0,0 Reuse Requested (ML/yr),0,0,0,0,0,0,0,0 % Reuse Demand Met,0,0,0,0,0,0,0,0 % Load Reduction,0,0,0,0,0,0,0,0 TSS Flow In (kg/yr),44310.9,13254,1828.14,27400.2,5052.86,5180.39,8045.05,6352.52 TSS ET Loss (kg/yr),0,0,0,0,0,0,0,0 TSS Infiltration Loss (kg/yr),0,0,0,0,0,0,0,0 TSS Low Flow Bypass Out (kg/yr),0,0,0,0,0,0,0,0 TSS High Flow Bypass Out (kg/yr),0,0,0,0,0,0,0,0 TSS Orifice / Filter Out (kg/yr),0,0,0,0,0,0,0,0 TSS Weir Out (kg/yr),0,0,0,0,0,0,0,0 TSS Transfer Function Out (kg/yr),13293.3,3976.2,548.442,8220.06,1515.86,1554.12,2413.52,1905.76 TSS Reuse Supplied (kg/yr),0,0,0,0,0,0,0,0 TSS Reuse Requested (kg/yr),0,0,0,0,0,0,0,0 TSS % Reuse Demand Met, 0, 0, 0, 0, 0, 0, 0, 0



TSS % Load Reduction, 70, 70, 70, 70, 70, 70, 70, 70, 70 TP Flow In (kg/yr),60.6672,18.0115,2.48434,35.7454,6.62579,6.79181,10.889,8.33642 TP ET Loss (kg/yr),0,0,0,0,0,0,0,0 TP Infiltration Loss (kg/yr),0,0,0,0,0,0,0,0 TP Low Flow Bypass Out (kg/yr),0,0,0,0,0,0,0,0 TP High Flow Bypass Out (kg/yr),0,0,0,0,0,0,0,0 TP Orifice / Filter Out (kg/yr),0,0,0,0,0,0,0,0 TP Weir Out (kg/yr),0,0,0,0,0,0,0,0 TP Transfer Function Out (kg/yr),42.467,12.608,1.73904,25.0219,4.63806,4.75427,7.62229,5.8355 TP Reuse Supplied (kg/yr),0,0,0,0,0,0,0,0 TP Reuse Requested (kg/yr),0,0,0,0,0,0,0,0,0 TP % Reuse Demand Met, 0, 0, 0, 0, 0, 0, 0, 0 TP % Load Reduction, 30.0001, 30, 30.0001, 29.9998, 29.9999, 29.9999, 29.9999, 30 TN Flow In (kg/yr),640.479,191.049,26.3516,392.58,72.6754,79.5227,115.946,97.4266 TN ET Loss (kg/yr),0,0,0,0,0,0,0,0 TN Infiltration Loss (kg/yr),0,0,0,0,0,0,0,0 TN Low Flow Bypass Out (kg/yr),0,0,0,0,0,0,0,0 TN High Flow Bypass Out (kg/yr),0,0,0,0,0,0,0,0 TN Orifice / Filter Out (kg/yr),0,0,0,0,0,0,0,0 TN Weir Out (kg/yr),0,0,0,0,0,0,0,0 TN Transfer Function Out (kg/yr),640.479,191.049,26.3516,392.58,72.6754,79.5227,115.946,97.4266 TN Reuse Supplied (kg/yr),0,0,0,0,0,0,0,0 TN Reuse Requested (kg/yr),0,0,0,0,0,0,0,0 TN % Reuse Demand Met, 0, 0, 0, 0, 0, 0, 0, 0 TN % Load Reduction,0,0,0,0,0,0,0,0 GP Flow In (kg/yr),8398.47,2495.45,344.2,5331.18,996.848,1059.15,1594.96,1281.81 GP ET Loss (kg/yr),0,0,0,0,0,0,0,0 GP Infiltration Loss (kg/yr),0,0,0,0,0,0,0,0 GP Low Flow Bypass Out (kg/yr),0,0,0,0,0,0,0,0 GP High Flow Bypass Out (kg/yr),0,0,0,0,0,0,0,0 GP Orifice / Filter Out (kg/yr),0,0,0,0,0,0,0,0 GP Weir Out (kg/yr),0,0,0,0,0,0,0,0 GP Transfer Function Out (kg/yr),167.969,49.9089,6.884,106.624,19.937,21.183,31.8992,25.6363 GP Reuse Supplied (kg/yr),0,0,0,0,0,0,0,0 GP Reuse Requested (kg/yr),0,0,0,0,0,0,0,0 GP % Reuse Demand Met, 0, 0, 0, 0, 0, 0, 0, 0 Other nodes Location, Pre-Development Node, Post-Development Node, nth-out, sth-out ID,2,3,4,5

Node Type, PreDevelopmentNode, PostDevelopmentNode, JunctionNode, JunctionNode IN - Mean Annual Flow (ML/yr),149,495,266,230 IN - TSS Mean Annual Load (kg/yr),16.1E3,2.11E3,1.23E3,883 IN - TP Mean Annual Load (kg/yr), 32.2, 59.0, 32.2, 26.8 IN - TN Mean Annual Load (kg/yr),304,347,194,153 IN - Gross Pollutant Mean Annual Load (kg/yr),160,20.5,6.88,13.6 OUT - Mean Annual Flow (ML/yr),149,495,266,230 OUT - TSS Mean Annual Load (kg/yr), 16.1E3, 2.11E3, 1.23E3, 883 OUT - TP Mean Annual Load (kg/yr), 32.2, 59.0, 32.2, 26.8 OUT - TN Mean Annual Load (kg/yr), 304, 347, 194, 153 OUT - Gross Pollutant Mean Annual Load (kg/yr),160,20.5,6.88,13.6 % Load Reduction, 0.00, 12.2, 10.7, 13.9 TSS % Load Reduction, 0.00, 98.1, 97.9, 98.3 TN % Load Reduction,0.00,78.6,77.4,80.0 TP % Load Reduction, 0.00, 60.6, 60.3, 61.0 GP % Load Reduction, 0.00, 99.9, 99.9, 99.9

#### Links

Location, Drainage Link, Drainage Link Source node ID,1,4,5,6,12,13,14,15,16,18,19,20,21,22,17,24,23,25,7,26,8,27,9,28,10,29,11,30 Target node ID,2,3,3,5,5,5,5,5,5,5,4,21,21,22,4,24,18,25,4,26,12,27,13,28,14,29,15,30,16 Muskingum-Cunge Routing, Not Routed, Not Routed Muskingum K, , , , , , , , , , , , , , , , , Muskingum theta, , IN - Mean Annual Flow (ML/yr),149,266,230,7.41,117,21.1,21.8,33.8,28.2,58.7,84.7,138,222,198,66.0,66.0,9.11,9.11,135,135,25.0 ,25.0,26.6,26.6,40.0,40.0,32.5,32.5



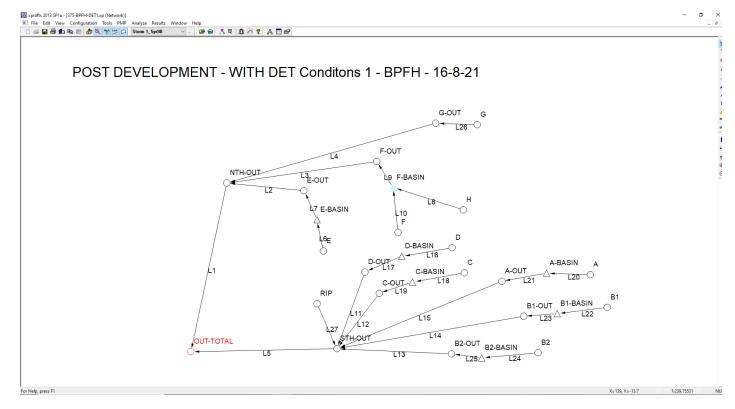
IN - TSS Mean Annual Load (kg/yr),16.1E3,1.23E3,883,308,305,55.6,50.7,89.2,73.6,155,17.0E3,27.3E3,13.3E3,524,13.3E3,3.98E3,1.83E3 ,548,27.4E3,8.22E3,5.05E3,1.52E3,5.18E3,1.55E3,8.05E3,2.41E3,6.35E3,1.91E3 IN - TP Mean Annual Load (kg/yr),32.2,32.2,26.8,0.364,14.0,2.52,2.60,4.03,3.35,6.97,23.1,37.6,42.5,23.5,18.0,12.6,2.48,1.74,35.7 ,25.0,6.63,4.64,6.79,4.75,10.9,7.62,8.34,5.84 IN - TN Mean Annual Load (kg/yr), 304, 194, 153, 7.39, 77.0, 13.9, 13.8, 22.2, 18.6, 38.4, 245, 395, 640, 129, 191, 191, 26.4, 26.4, 393, 393, 72.7, 7 2.7,79.5,79.5,116,116,97.4,97.4 IN - Gross Pollutant Mean Annual Load (kg/yr),160,6.88,13.6,13.6,0.00,0.00,0.00,0.00,0.00,0.00,3.20E3,5.20E3,168,0.00,2.50E3,49.9,344,6.88,5. 33E3,107,997,19.9,1.06E3,21.2,1.59E3,31.9,1.28E3,25.6 OUT - Mean Annual Flow (ML/yr),149,266,230,7.41,117,21.1,21.8,33.8,28.2,58.7,84.7,138,222,198,66.0,66.0,9.11,9.11,135,135,25.0 ,25.0,26.6,26.6,40.0,40.0,32.5,32.5 OUT - TSS Mean Annual Load (kg/yr),16.1E3,1.23E3,883,308,305,55.6,50.7,89.2,73.6,155,17.0E3,27.3E3,13.3E3,524,13.3E3,3.98E3,1.83E3 ,548,27.4E3,8.22E3,5.05E3,1.52E3,5.18E3,1.55E3,8.05E3,2.41E3,6.35E3,1.91E3 OUT - TP Mean Annual Load (kg/yr), 32.2, 32.2, 26.8, 0.364, 14.0, 2.52, 2.60, 4.03, 3.35, 6.97, 23.1, 37.6, 42.5, 23.5, 18.0, 12.6, 2.48, 1.74, 35.7 ,25.0,6.63,4.64,6.79,4.75,10.9,7.62,8.34,5.84 OUT - TN Mean Annual Load (kg/yr),304,194,153,7.39,77.0,13.9,13.8,22.2,18.6,38.4,245,395,640,129,191,191,26.4,26.4,393,393,72.7,7 2.7,79.5,79.5,116,116,97.4,97.4 OUT - Gross Pollutant Mean Annual Load (kg/yr),160,6.88,13.6,13.6,0.00,0.00,0.00,0.00,0.00,0.00,3.20E3,5.20E3,168,0.00,2.50E3,49.9,344,6.88,5. 33E3,107,997,19.9,1.06E3,21.2,1.59E3,31.9,1.28E3,25.6

#### Catchment Details

Catchment Name,bp-forest hill-ver1-5-8-21 Timestep,Day Start Date,1/01/1968 End Date,31/12/1977 Rainfall Station, 70014 CANBERRA ET Station,User-defined monthly PET Mean Annual Rainfall (mm), 655 Mean Annual ET (mm), 1116 MUSIC-link Area, ACT Government MUSIC-link Scenario, ACT Development



# **APPENDIX C (RAFTS)**



xprafts 2013 SP1a - [375-BPFH-DET1.xp:2 (Review Results)]

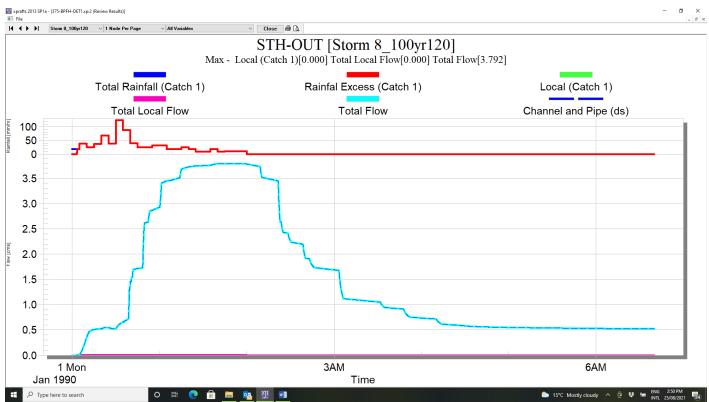
| ◀ ◀ ▶ ▶| Storm 8\_100yr120 ∨ 1 Node Per Page All Variables Close 🖨 🛕 NTH-OUT [Storm 8\_100yr120] Max - Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[3.963] Total Rainfall (Catch 1) Rainfal Excess (Catch 1) Local (Catch 1) Total Local Flow Total Flow Channel and Pipe (ds) 100 50 0 4.0 3.5 3.0 2.5 2.0 1.5 1.0 0.5 0.0 1 Mon 3AM 6AM Time Jan 1990 Type here to search 

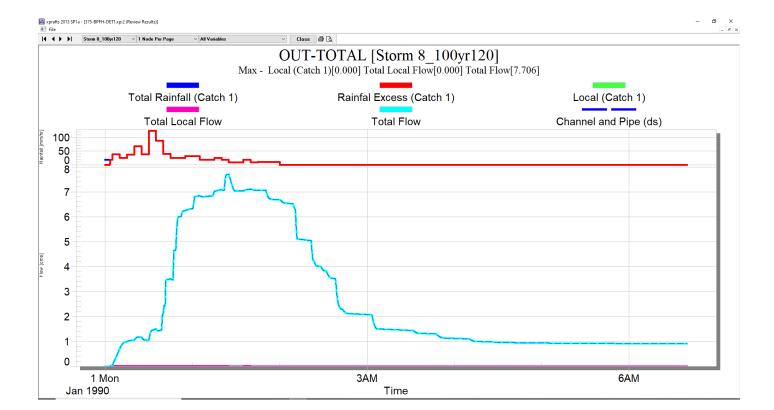
o 🖽 💽 📅 📻

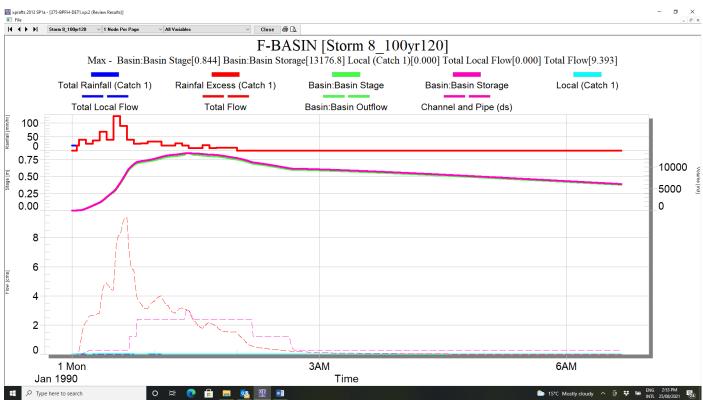
02

w

-

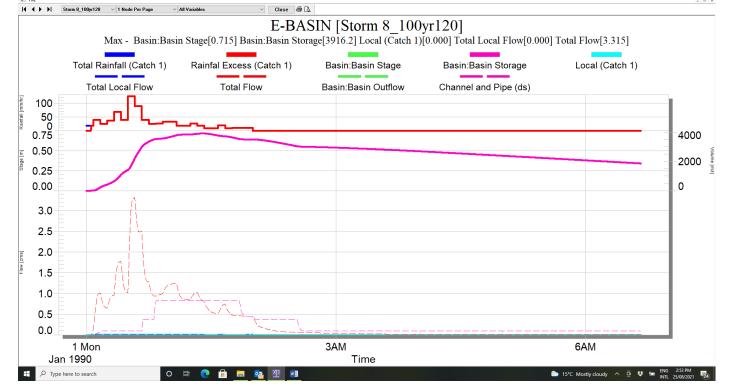


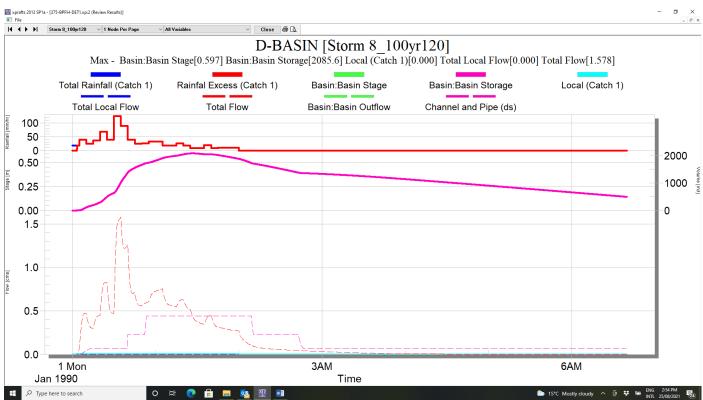




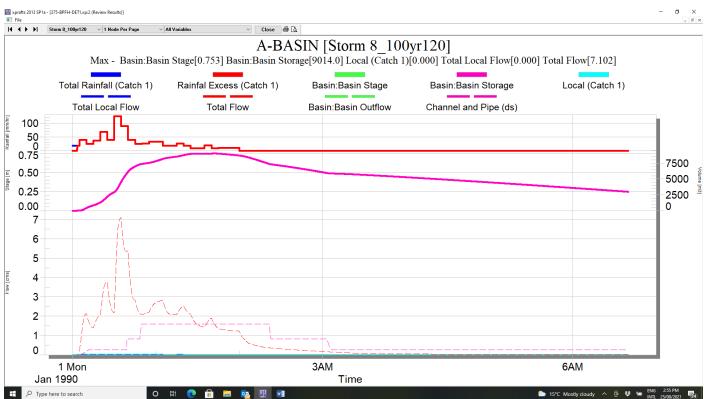
#### xprafts 2013 SP1a - [375-BPFH-DET1.xp:2 (Review Results)

ð × - 8 ×



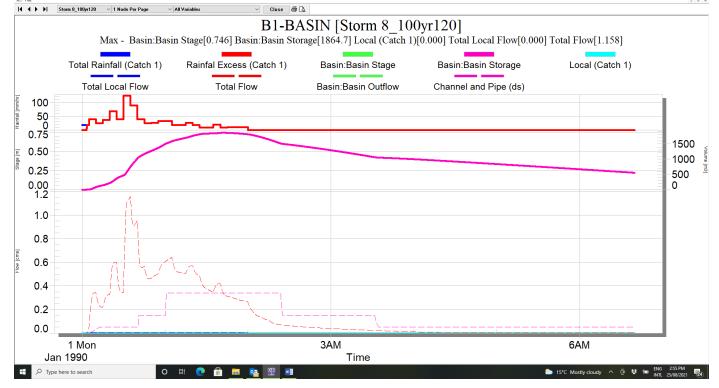


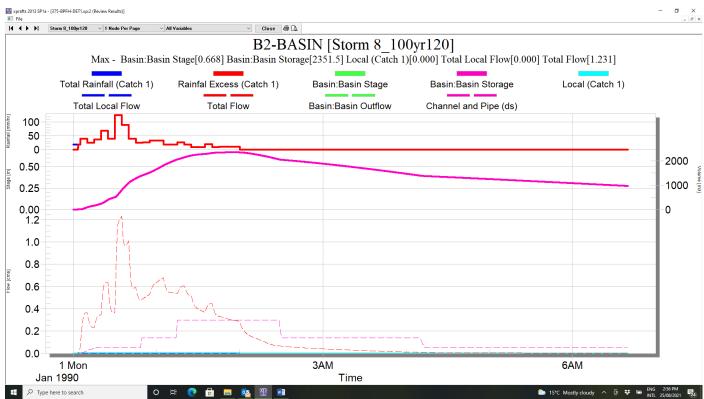
#### 8 × xprafts 2013 SP1a - [375-BPFH-DET1.xp:2 (Review Res [◀ ◀ ▶ ▶] Storm 8\_100yr120 ∨ 1 Node Per Page Close 🖨 🕅 All Variab C-BASIN [Storm 8\_100yr120] Max - Basin:Basin Stage[0.692] Basin:Basin Storage[3118.9] Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[1.852] Basin:Basin Storage Total Rainfall (Catch 1) Rainfal Excess (Catch 1) Local (Catch 1) Basin:Basin Stage Total Local Flow Total Flow Basin:Basin Outflow Channel and Pipe (ds) 100 50 antal 0 3000 0.50 2000 stage Imi 0.25 1000 0.00 0 1.5 1.0 0.5 0.0 3AM 1 Mon 6AM Jan 1990 Time . $\,\mathcal{P}\,$ Type here to search 😻 📾 ENG

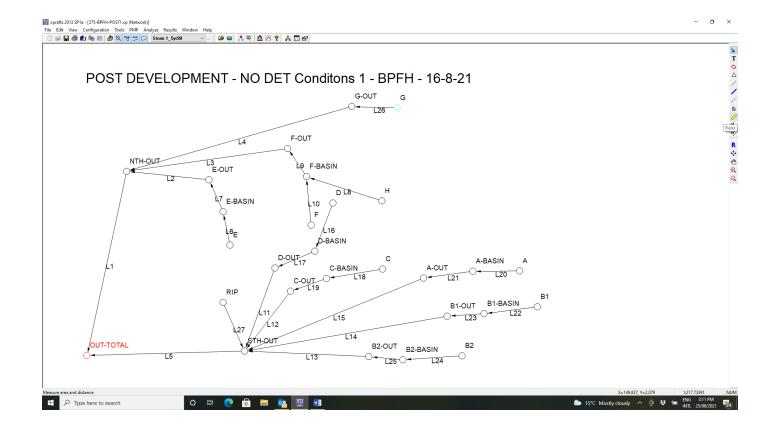


#### 🗱 xprafts 2013 SP1a - [375-BPFH-DET1.xp:2 (Review Results)]

- 0 ×

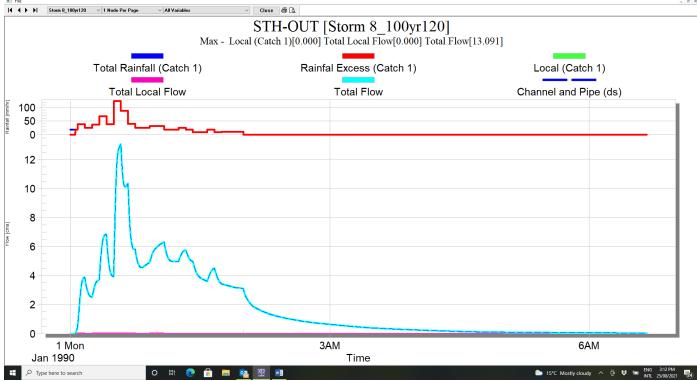


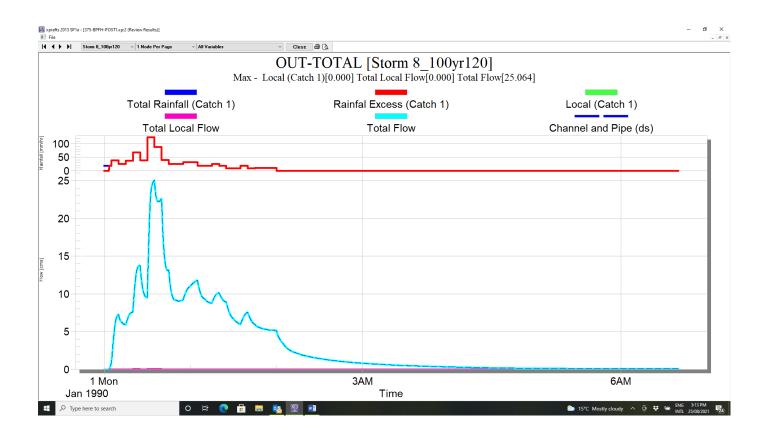




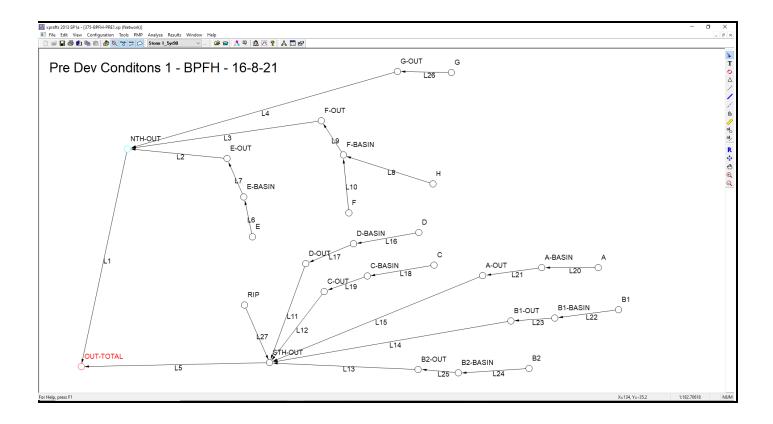
#### wprafts 2013 SP1a - [375-BPFH-POST1.xp:2 (Review Results)]

- 0 × Close 🖨 👌 ~ All Variables NTH-OUT [Storm 8 100yr120] Max - Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[12.264] Total Rainfall (Catch 1) Rainfal Excess (Catch 1) Local (Catch 1) Total Flow **Total Local Flow** Channel and Pipe (ds) 100 50 0 12 10 8 6 4 2 0 3AM 6AM 1 Mon Jan 1990 Time 🗄 🔎 Type here to search ● 15\*C Mostly cloudy へ 逆 撃 📟 ENG 3:12 P 最 n 🔁 💼 💁 🔛





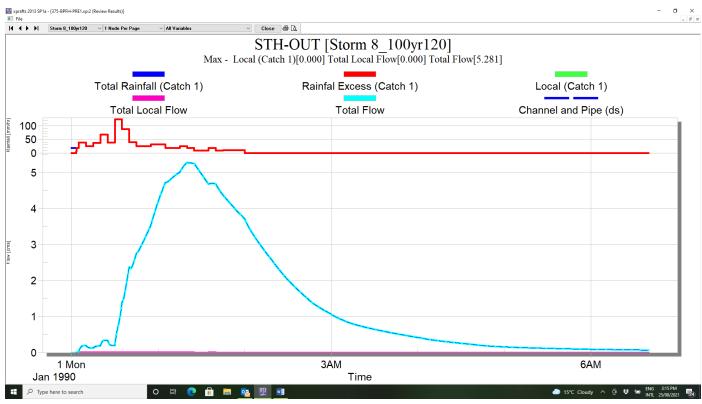
# - 0 ×



#### xprafts 2013 SP1a - [375-BPFH-PRE1.xp:2 (Review Results)]

× I I Node Per Page All Variables Close 🖨 👌 NTH-OUT [Storm 8\_100yr120] Max - Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[3.937] Rainfal Excess (Catch 1) Total Rainfall (Catch 1) Local (Catch 1) Total Flow Total Local Flow Channel and Pipe (ds) 100 50 0 4.0 3.5 3.0 2.5 2.0 1.5 1.0 0.5 0.0 6AM 3AM 1 Mon Jan 1990 Time Type here to search 😻 📾 ENG 朂

٥ -



#### - 0 × wprafts 2013 SP1a - [375-BPFH-PRE1.xp:2 (Review Results)] Close 🖨 🗟 [◀ ◀ ▶ ▶] Storm 8\_100yr120 ∨ 1 Node Per Page All Variables OUT-TOTAL [Storm 8\_100yr120] Max - Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[9.197] Total Rainfall (Catch 1) Rainfal Excess (Catch 1) Local (Catch 1) Total Local Flow Total Flow Channel and Pipe (ds) 100 50 0 9 8 7 6 5 4 3 2 1 0 3AM 6AM 1 Mon Jan 1990 Time

#### LOCATION 35.150 S 147.450 E \* NEAR.. Forest Hill NSW

#### LIST OF COEFFICIENTS TO EQUATIONS OF THE FORM

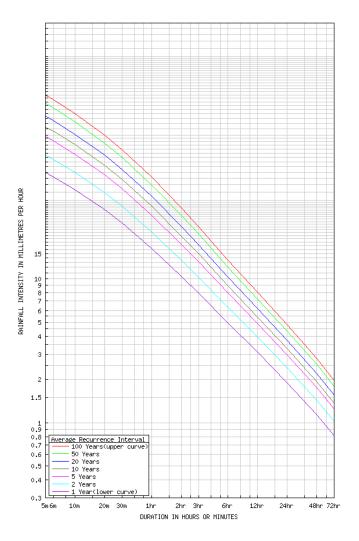
#### $ln(l) = A + B \times (ln(T)) + C \times (ln(T))^{2} + D \times (ln(T))^{3} + E \times (ln(T))^{4} + F \times (ln(T))^{5} + G \times (ln(T))^{6}$

RETURN PERIOD	A	В	С	D	E	F	G
1	2.787071	-0.62043E+0	-0.40485E-1	0.10178E-1	0.52410E-3	-0.56242E-3	0.37213E-4
2	3.051620	-0.62757E+0	-0.41469E-1	0.10143E-1	0.60652E-3	-0.54424E-3	0.31510E-4
5	3.323019	-0.64592E+0	-0.45984E-1	0.10017E-1	0.11470E-2	-0.50339E-3	0.78125E-5
10	3.461665	-0.65607E+0	-0.47302E-1	0.10043E-1	0.11981E-2	-0.49209E-3	0.66898E-5
20	3.620587	-0.66481E+0	-0.49389E-1	0.10125E-1	0.14416E-2	-0.49020E-3	-0.14632E-5
50	3.804105	-0.67465E+0	-0.51202E-1	0.10143E-1	0.15915E-2	-0.48180E-3	-0.62248E-5
100	3.929076	-0.68110E+0	-0.52937E-1	0.99153E-2	0.18481E-2	-0.44005E-3	-0.22125E-4

#### RAINFALL INTENSITY IN mm/h FOR VARIOUS DURATIONS AND RETURN PERIODS

RETURN PERIOD (YEARS)							
DURATION	1	2	5	10	20	50	100
5 mins	54.9	72.3	97.9	114.	136.	166.	190.
6 mins	51.1	67.4	90.9	106.	126.	154.	176.
10 mins	41.6	54.7	73.4	85.6	101.	123.	141.
20 mins	30.2	39.6	52.8	61.2	72.3	87.6	99.8
30 mins	24.4	31.9	42.3	48.9	57.7	69.7	79.3
1 hour	16.2	21.1	27.7	31.9	37.4	44.9	50.9
2 hours	10.4	13.5	17.4	19.8	23.1	27.5	31.0
3 hours	7.93	10.2	13.1	14.9	17.2	20.4	22.9
6 hours	4.95	6.36	7.99	8.98	10.3	12.1	13.5
12 hours	3.09	3.94	4.88	5.44	6.22	7.25	8.05
24 hours	1.90	2.42	2.97	3.29	3.75	4.34	4.81
48 hours	1.13	1.43	1.75	1.93	2.20	2.54	2.81
72 hours	.809	1.02	1.24	1.37	1.55	1.79	1.97

(Raw data: 21.63, 4.00, 1.03, 43.50, 7.01, 1.73,skew= 0.200) HYDROMETEOROLOGICAL ADVISORY SERVICE (C) AUSTRALIAN GOVERNMENT, BUREAU OF METEOROLOGY \* ENSURE THE COORDINATES ARE THOSE REQUIRED SINCE DATA IS BASED ON THESE AND NOT LOCATION NAME.





# APPENDIX D (CV)



Civil Certification Pty Ltd Accredited Certifiers

Civil Engineering ABN 56 607 721 595 0412 264 237



#### Michael J Shaw BE MIEAust CPEng NER

Director Civil Certification Pty Ltd

Resume

## 1. SUMMARY

Michael is a senior civil engineer with over 26 years' experience in the fields of civil engineering, road design, drainage, hydrology, stormwater management and urban infrastructure design. He operates his own business specialising in subdivision certification and stormwater management. Michael has worked on many civil design projects ranging from development of large scale strategic masterplans to detailed design of stormwater management facilities and urban infrastructure for residential subdivisions. His expertise lies in solving complicated drainage problems, water sensitive urban design (*WSUD*), erosion & sediment control, flooding, detailed civil design, understanding the local government approvals process and managing multidisciplinary teams. Michael's experience covers all facets of civil engineering for urban development from due diligence through to approvals, detailed design, superintendency and certification. He has also provided expert advice to the Land and Environment Court with relation to flooding, drainage, riparian, detention and stormwater quality issues.

## 2. **EXPERIENCE**

#### **Positions Held -& Location**

Oct 2010 – Present	Director, Civil Certification Pty Ltd, Sydney, NSW, Australia			
April 2008- Sept 2010	Manager, Urban Infrastructure, Environment Group - Worley Parsons, Sydney, NSW, Australia.			
Aug. 2007- March 2008	<ul> <li>Principal Engineer – Urban Infrastructure - Worley Parsons incorporating Patterson Britton &amp; Partners, Sydney, NSW, Australia;</li> </ul>			
Nov. 1997- Jul. 2007	Senior Associate – Urban Infrastructure - Patterson Britton & Partners, Sydney, NSW, Australia;			
Aug. 1996- Oct 1997	Water Resources Engineer – Willing & Partners, Sydney, NSW, Australia;			
Feb. 1991- Aug. 1994	Design Engineer, Development Engineer, Investigation Engineer & Survey Assistant – Ryde City Council, Sydney, NSW, Australia.			
	Standout Projects			
	Stormwater Management Strategies(SMS)			
	- Port Jackson South Stormwater Management Plan (2,870ha catchment);			
	- Drummoyne Council Stormwater Quality Strategy (830ha catchment);			
	- Lake Illawarra South Stormwater Quality Strategy (1,548ha catchment);			
	- Elliot Lake Stormwater Quality Strategy (1,220ha catchment);			

1

- Scotland Island SMS (53ha catchment);
- Corks Lane Milton, DA Stage SMS (150 lot residential subdivision);



Accredited Certifiers Civil Engineering ABN 56 607 721 595

0412 264 237



#### Michael J Shaw BE MIEAust CPEng NPER

Director Civil Certification Pty Ltd

## Resume

- Pasadena, Church Point, DA Stage Stormwater Management and Reuse Strategy (mixed use dev.);
- Yallambee Ave West Gosford, DA Stage SMS (100 lot residential subdivision);
- CSIRO Greystanes, Employment Lands SMS (60ha industrial site);
- Warriewood Valley, Sector 3, Rezoning Stage SMS (130 lot residential subdivision);
- Warriewood Valley, Sector 8, Rezoning to Subdivision Certificate Stage SMS (140 lot residential subdivision);
- Warriewood Valley, Buffer Areas 1 and 2, Rezoning and DA Stage SMS (*300 lot residential subdivision;*
- Warriewood Valley, Buffer Area 3, Rezoning and DA Stage SMS (250 townhouse subdivision);
- Macarthur Square Regional Centre Masterplan DA Stage WSUD Strategy (61ha residential subdivision);
- Department of Defence Site, Ermington ("*Ermington Riverfront*") DA Stage SMS (20ha residential subdivision;
- West Kembla Grange, Wollongong, Aquatic Issues Assessment (858ha catchment);
- Eastwood Quarry, Masterplan/Rezoning Stage SMS (20ha residential subdivision);
- Perentie and Dawes Road Masterplan, Belrose, Stormwater Quality Strategy (*30ha residential subdivision*);
- Walter Road, Ingleside DA Stage SMS (15ha rural residential subdivision);
- Domayne, Austlink Park Belrose SMS (large commercial use development);
- Grassmere LES, Camden SMS (50ha rural residential subdivision);
- Warriewood Valley (Sectors C, D, & 12) Rezoning Stage SMS (100 lot residential subdivision);
- -Summer Hill Flour Mill Concept Plan Application Stormwater Management Plan and Flood Study (250 dwelling high density residential subdivision);
- Mt Penang Stormwater Management Strategy;
- Ashlar Golf Course Redevelopment Flood and WSUD Strategy for 100 dwelling Residential Subdivision;
- Water Sensitive Urban Design (WSUD)
- Sand Filtration Unit, Drummoyne Park (ie Stormwater Treatment);
- Barnwell Park Golf Course Stormwater Treatment and Reuse;
- Powell Creek Reserve Eco Carpark;
- Warriewood Valley, Sector 10, Detailed Design of WSUD elements (*bio-retention systems and wetland for 170 lot residential subdivision*);
- Warriewood Valley, Sector 12, Detailed Design of WSUD elements (*bio-retention systems and wetland for 180 lot residential subdivision*);
- Rouse Hill Regional Centre Detailed design and performance analysis of bio retention systems, raingardens and constructed wetland;



Accredited Certifiers Civil Engineering

ABN 56 607 721 595

0412 264 237



#### Michael J Shaw BE MIEAust CPEng NPER

Director Civil Certification Pty Ltd

#### Resume

- Hezlett Road, North Kellyville Generic lot based raingarden design, road bio-retention swale design and detention offset analysis;
- Voyager Point (*DHA*) Detailed Design of Detention/Bio-Retention Basins for 200 lot residential subdivision;

#### Riparian/Creek Design/Investigation

- Wollondilly Shire Riparian Corridor Definition Study;
- Parsley Bay, Woollahra, Creekline Rehabilitation;
- Embankment Stabilisation Design, Koloona Ave, Byarong Creek ,Wollongong;
- Embankment Stabilisation Design, 5 sites along Cabbage Tree Creek, Towradgi Creek and Byarong Creek, Wollongong;
- Prospect Creek, Fairfield Design of confluence stabilisation and creek rehabilitation measures;
- Narrabeen Creek, Pittwater Detailed design of creek rehabilitation and embankment stabilisation measures from Graf Ave to Ponderosa Parade;
- Little Bay Central Drainage Corridor Controlled Activity Approval and detailed design of Central Corridor Drainage Features (*ie wetlands, bio-retention basins, weirs, elevated walkways, bridges, pool/riffle creekline*);
- Sector 8 Warriewood Controlled Activity Approval for residential development adjoining Fern Creek;
- Nolan's Reserve Footbridge Abutment Armour and bridge repair design;
- Middle Creek Footbridge Armour design (Narrabeen Lakes for Warringah Council);

#### Civil Subdivision Design

- Potts Hill, Eastern Precinct Lead design team for 13ha Industrial development of Sydney Water Surplus Lands. Engaged by Landcom to provide approval documentation for Part 3 Major Project and to deliver detailed design of all subdivision infrastructure (*ie civil, roads, E&S control, RE walls,* stormwater, power, sewer, water, recycled water and utility services);
- Tweed Road, Lithgow, Detailed Design of Civil Infrastructure (*roads, drainage, E&S control, water, sewer and all other utility services*) for a 38 lot residential subdivision;
- Sector 20, Warriewood, Detailed Design of Civil Infrastructure (*roads, drainage, E&S control, water, sewer and all other utility services*) for a 63 lot residential subdivision;
- 7 Orchard Road, Warriewood, Detailed Design of Lot Based Stormwater Management Facilities and Access Road for a 10 lot residential subdivision;
- Heritage Estates, Shoalhaven, Conceptual Design of Civil Infrastructure. (*water, sewage, utility services, roads and drainage*) for 20ha residential subdivision;
- Randwick Defence Site (*Stage 1A*), Detailed Design of Civil Infrastructure (*roads, drainage, water, sewer and all other utility services*) for a 80 lot residential subdivision;



Accredited Certifiers Civil Engineering

ABN 56 607 721 595

0412 264 237



#### Michael J Shaw BE MIEAust CPEng NPER

Director Civil Certification Pty Ltd

## Resume

 Cooks Cove Development, Upgrade to Scarborough and Bicentennial Parks – Lead design team for approvals and detailed design of upgrade to park facilities, including carparks, creekline, stormwater drainage, bulk earthworks, access roads, services, *E&S control* etc to accommodate future relocation of facilities from Cooks Cove development site (*Part 3A Major Project*);

#### Drainage Analysis/Design

- Canada Bay Council city wide DRAINS modelling project (970ha catchment);
- Canada Bay Council Detention modelling and OSD policy development;
- City of Canada Bay Council MAPINFO drainage database update;
- Old Bathurst Road, Emu Plains, Detailed Design of Stormwater Management Facilities (24ha industrial subdivision);
- Andrew Road, Penrith, Detailed Design of Stormwater Management Facilities (8ha industrial subdivision);
- St Mervyns Ave, Woollahra, Stormwater Outlet Extension;
- Grosvenor Street Stormwater Drainage Study;
- Perentie and Dawes Road Masterplan, Belrose, Stormwater Drainage Concept Plan;
- Yulong Concept Drainage Study, Dept Defence Moorebank (25ha industrial subdivision);
- Headland Road, Curl Curl OSD Design;
- Cooper Park Amphitheatre , Woollahra, Detailed Stormwater Drainage Design;
- Paradise Avenue, Paradise Beach, Detailed Stormwater Drainage Design;
- Georges River Sailing Club, Seawall and Beach Nourishment Design;
- St Andrew Church, Wahroonga OSD and Stormwater Drainage Design;
- North Sydney Catchment Management Studies (in total 86ha catchment);
- Greystanes Estate, Northern Residential Lands, Detailed Design of Water Management Facilities (70ha residential development);

4

- Barina Downs Road, Detention Basin Design (large regional detention facility);
- Robertson Road, Scotland Island Detailed Stormwater Drainage Design;
- Jenkins Road, Dundas Detention System Design;
- Lot 2 Muir Road, Chullora Drainage and Detention System Design for Large Industrial Development;

#### Flood Studies (FS)

- Prospect Creek Channel Enhancement FS;
- Oats Ave, Gladesville FS;
- Casa Paloma Caravan Park FS;
- Kiaora Place Development, Double Bay FS;
- Darling Park/Cross City Tunnel Flood impact assessment;
- Mowbray Road, Nursing Home, Assessment of overland flow impacts;
- Macquarie Links Golf Course FS (Bunburry Curran Creek, Campbelltown);



Accredited Certifiers Civil Engineering

ABN 56 607 721 595

0412 264 237



#### Michael J Shaw BE MIEAust CPEng NPER

Director Civil Certification Pty Ltd

#### Resume

- Wigan Road, Dee Why FS;
- Green Road FS;
- Anzac Creek, Moorebank FS;
- Eastwood Hotel Drainage/Flooding Study;
- Mona Street, Mona Vale FS;
- Frenchs Creek FS;
- Darling Walk 2D TUFLOW Flood Assessment, Darling Harbour;
- Lynwood Ave, Dee Why Flood Assessment;
- Ashlar Golf Course Development 2D TUFLOW Flood Study;
- Ashlar Golf Course Development HEC RAS Flood Modelling Breakfast Creek and Western Channel;

#### Dam Hazard Assessment

- Kellyville Ridge Dam, Second Ponds Creek, Dam Hazard Assessment;
- UWS Campbelltown Dam Hazard Assessment;
- Hume Golf Course, Albury Dam Hazard Assessment;
- CSIRO, Greystanes Dam Hazard Assessment;
- Honeysuckle Creek Dam DSC Surveillance Report (Killara Golf Course);

#### Water Quality Monitoring

- Sectors 2, 8 and 11 Warriewood, Post construction (*ie residential subdivision*) stormwater quality monitoring;
- Warriewood Valley (Various Sectors) Approval Stage Water Quality Monitoring over an 8 year period
- Shellharbour Council Stormwater Monitoring Strategy (entire Shellharbour LGA 14,000ha);
- St Marys Eastern Precinct Water Quality Monitoring Strategy (160ha residential subdivision);
- Rouse Hill Regional Centre Post development Water Quality Monitoring of treatment measures and receiving waters (*Auto sampling and Grab sampling*);
- Water Quality Sampling for Metal Recycling development, Ingleburn.
- 2 Year Surface Water and Bed Sediment Monitoring Program, Ashlar Residential Development, Blacktown
- -Sector 3, WWV Residential Development Pre and Post Development Development Water Quality Monitoring;
- -Aussie Skips, Strathfield EPA Water Quality Monitoring Program.

#### Major Culvert Amplification Design

- Careel Creek/Barrenjoey Road Culvert Amplification Works (Pittwater Council and RTA);
- Nareen Creek /Narrabeen RSL Culvert Entry Upgrade (Pittwater Council);
- Howell Reserve Culvert Entry Upgrade and Drainage Diversion Line (Pittwater Council);
- Fern Creek/Garden Street Culvert Amplification (Pittwater Council);



Accredited Certifiers Civil Engineering ABN 56 607 721 595

0412 264 237



#### Michael J Shaw BE MIEAust CPEng NPER

Director Civil Certification Pty Ltd

## Resume

- Narrabeen Creek/Ponderosa Pde Culvert Amplification (Pittwater Council);
- Garie Beach Culvert Amplification (RTA and NPWS);
- Bridge Amplification Design, Narroy Road, Narrabeen.

#### Road/Carpark Design

- Transport Infrastructure Development Corporation (*TIDC*) Commuter Car Park Program, Detailed Design of At Grade Carparks at Emu Plains Station, Woonona Station and Waterfall Station;
- Rookwood Road Potts Hill, Detailed Design of RTA signalised intersection upgrade (Landcom)
- Brunker Road Potts Hill, Detailed Design of RTA signalised intersection upgrade (Landcom)
- Scotland Island Road Reserve Masterplan (53ha area);
- P&O Port Botany, Detailed Design of Staff Carparking Facilities (50 spaces);
- McKeown Street, Maroubra Beach, Detailed Road Design for streetscape improvement works;
- Department of Defence Site, Randwick (*Stages 1A, 1B and Community Centre*), Detailed Road Design for large residential subdivision(*5.6ha residential subdivision*);
- Greystanes Estate Northern Residential Land, Detailed Road Design for large residential subdivision (50ha residential subdivision);
- Sector 20 Warriewood, Detailed Road Design for large residential subdivision (50ha residential subdivision);
- Lidcombe Botanica, Detailed Road Design for heritage precinct of large residential subdivision;
- Heffron Park Randwick, Detailed Design of 100 space carpark and associated road improvement works.

#### Infrastructure/Servicing Strategies

- Ermington Naval Stores (700 lot residential development);
- Greystanes Estate, Prospect (250ha residential & employment development);
- UWS Werrington (48ha residential development);
- Airds Town Centre Masterplan;
- Sector 7 (2 Daydream Avenue), Warriewood (3ha mixed use commercial/light industrial development);
- St Mary's (ADI Site-Eastern Precinct-160ha residential development);
- Green Square Master Plan, South Sydney (Zetland);
- Mt Penang, Gosford Business Park development.

#### ▶ Gross Pollutant Traps (GPT's)

- Dee Why Beach GPT design (special non proprietary);
- Birkenhead Point and Brent Street GPTs (special non proprietary);
- St Georges Crescent Catchment Oil/Grit Separators (multiple proprietary);
- Stormwater Trust Application Assistance, Waterways Authority Blackwattle Bay GPT;
- Brookvale Creek Rehabilitation detailed design of large offline GPT/trash rack;



**Civil Certification Pty Ltd** Accredited Certifiers Civil Engineering

ABN 56 607 721 595

0412 264 237



## Michael J Shaw BE MIEAust CPEng NPER

Director Civil Certification Pty Ltd

## Resume

- Drummoyne Council - Three Ways to Improve The Bays GPT Design Project (*special non-proprietary*).

#### General Civil Engineering

- BER Sydney South, provision of general civil engineering design for Abigroup for a number of Schools in Sydney South;
- McCarr's Creek Road/Pittwater Road Inventory and Condition Assessment;
- Design of steel pedestrian bridge, elevated walkway, stairs and reinforced concrete weirs for residential subdivision at Little Bay;
- Design of playground equipment footings, BBQ shelter footings/slab and small reinforced concrete retaining walls for playground at Sector 8, Warriewood Valley;
- Seawall design 26 Prince Alfred Parade, Newport;
- Reinforced Earth retaining wall design for Sydney Water/Landcom, Potts Hill.

#### Expert Advice / L&E Court

- DA Stormwater management, West Ryde Urban Village Redevelopment for Ryde City Council (*ie acting on behalf of Council*);
- DA Stormwater management, Top Ryde Shopping Centre Redevelopment for Ryde City Council( *ie acting on behalf of Council*);
- Yulong Moorebank , review of road design for Department of Defence;
- Rushcutters Bay Flood Study Peer Review for Lindsay Bennelong Developments;
- Review of Managing Urban Stormwater Manual April 2004 on behalf of Landcom;
- Clontarf Street, Seaforth Civil inspections for Landcom res. dev. on behalf of Manly Council;
- Sector 20, Warriewood Superintendency for \$6 million Civil Works Contract;
- Expert witness (*water quality on industrial site*) for L&E Court Case Phiney Place, Ingleburn (*representing private developer*);
- Expert witness (*drainage, S88K easement*)) for L&E Court Case Park Street, Mona Vale (*representing adjoining land owner*);
- Expert witness (*drainage/absorption system/easement*) for L&E Court Case 120 Hopetoun Avenue, Vaucluse (*representing owner/developer*);
- Expert Witness (drainage, riparian, water quality, OSD) Sector 3, Warriewood Valley (Sunland Developments)
- Expert witness (*drainage/riparian corridor*) for L&E Court Case 23B Macpherson Street, Warriewood (*representing private developer*);
- Expert witness (*riparian matters/controlled activity application/culvert creek crossing*) for Supreme Court Case Wambo Coal Mine, Warkworth (*representing mine operator*);
- Expert Witness Anglican Church, Commercial Rd, Rouse Hill Drainage Matters;



Civil Certification Pty Ltd Accredited Certifiers Civil Engineering

ABN 56 607 721 595

0412 264 237



#### Michael J Shaw BE MIEAust CPEng NPER

Director Civil Certification Pty Ltd

## Resume

- Expert Witness Warringah Council, Development at Bantry Bay Rd, Forestville Stormwater Management/Water Quality and Detention;
- Expert Witness Abax Contracting, Development at 2 Fern Creek Road Warriewood Stormwater Drainage, Water Quality, Detention and Flooding.

#### Certification

- Subdivision PCA for Central Open Space Works, Cobaki (*Tweed Council LGA*) Part 3A Major Project ;
- Subdivision PCA Lachlan's Line, Macquarie Park (*NSW Urban Growth/Ryde Council LGA*) Part 3A Major Project;
- St Marys ADI Site Construction Certificate and Compliance Inspections for Bulk Earthworks, S&E Control and roads;
- Construction Certificate Issue and compliance inspections for Bulk Earthworks and Civil Infrastructure (*Roads & Drainage*), Precincts 1&2 (450 lots) and 6 Cobaki (450 lots) (*Tweed Council LGA*);
- Civil compliance inspections and Part4A certificates for Putney Hill Development, Ryde (100 lots)– Part 3A Major Project (*Ryde Council LGA*);
- Civil Compliance Inspections and certificates for Kiah Development (Stage 4), Willoughby (50 lots);
- Civil Compliance inspections and Part4A certificates for DHA Riverfront Development, Ermington (500 lots) Part 3A Major Project (DHA/*Parramatta Council LGA*);
- Civil Compliance inspections and Part4A certificates for Clemton Park Development (*Australand*) Part 3A Major Project (*Canterbury Council LGA*);
- Bunya Collector Road CC Private certification assessment for Landcom;
- Construction Certificate issue (*Private Certification*) for small subdivision at Mount Street, Constitution Hill;
- Construction Certificate issue (*Private Certification*) for small subdivision works at 28 Crescent Rd, Mona Vale;
- -Construction Certificate issue (*Private Certification*) for small subdivision works at Brush Rd, Eastwood;
- Part4A Compliance Certificate (Road and Drainage Works) for Retirement Village at Evans Road, Rooty Hill;
- Penrith Lakes Weir 3 CC assessment and compliance inspections for NSW Planning;
- Construction Certificate Drainage Works Little Street, Lane Cove;
- CC and Inspections Elara MD103 and Elara RL, Marsden Park (Stocklands);
- -CC Altrove Stage 7, Schofields (Stocklands);
- -CC and Inspections, Willowdale (Stocklands).



**Civil Certification Pty Ltd** Accredited Certifiers Civil Engineering ABN 56 607 721 595

0412 264 237



Michael J Shaw BE MIEAust CPEng NPER

Director Civil Certification Pty Ltd

Resume

# 3. EDUCATION & PROFESSIONAL AFFILIATIONS

- Bachelor of Engineering (*Civil*), University of Technology, Sydney, 1996;
- Member, Institution of Engineers, Australia (*MIEAust*);
- Charted Professional Engineer (CPEng);
- National Professional Engineers Register (NER Civil);
- NSW Accredited Certifier (BPAct 2005) Categories B1, C1, C2, C3, C4, C6, C7, C12, C15, C16 (BPB 0816)
- Member of BPB Disciplinary Committee (2015-2016)